University of Saint Joseph Athletic Field Renovations

West Hartford, Connecticut

Stormwater Management Report

February 20, 2019 Project # 19014



Submitted by: SMRT Architects and Engineers 877.700.7678 smrtinc.com



ARCHITECTURE ENGINEERING PLANNING INTERIORS ENERGY smrtinc.com

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1 PROJECT NARRATIVE

1.1 Introduction

The project consists of the construction of a new multi-sport synthetic turf field and upgraded field lighting. Athletics are an integral component of student life at Saint Joseph's, and the University is looking to enhance and upgrade its current amenities. The proposed project development area is located at the northwest end of campus, within the footprint of the existing track and field. The track and natural grass field are in poor condition, and the University intends to remove these amentities and replace them with one new, lighted, multi-purpose synthetic turf field to support the University's field hockey, soccer, and lacrosse programs.

The construction of the new synthetic turf field will occur entirely within previously disturbed areas. There will be no impact to surrounding trees, vegetation, or parking areas, and there will be no significant changes to actual land uses on the site. The project will result in alterations to existing cover conditions within the project area. This report describes the impact of the cover changes on surface runoff quantity and quality, and describes measures that have been incorporated into the design to ensure that there will be no detrimental impacts to downstream receiving waters.

In addition to permanent stormwater management measures, a comprehensive array of temporary soil erosion and sediment control measures (SESC) will be installed to serve the construction phase of the project. Disturbed areas of the site will be covered and stabilized as soon as practical to avoid exposure of bare soil. Sediment transport will be minimized through the use of barriers, diversions and other Best Management Practices. The SESC measures will be inspected and maintained throughout construction, and until final stabilization is achieved across the site.

As a result of these measures, the development is not expected to have any significant impacts on downstream water quality or quantity.

1.2 Site Location

The University of Saint Joseph is located at 1678 Asylum Avenue in West Harford, CT and is bounded by Albany Avenue to the north, Steele Road to the east, and Trout Brook Drive to the west. The project development area is located at the northwest end of the campus, due west of the O'Connell Athletic Center. Six (6) existing tennis courts are located to the north of the development area, an asphalt walkway lines the eastern edge, and parking areas are located to the south. The development area currently consists of a 400-meter synthetic surfaced running track, and a natural grass multi-sport athletic field.

See Figure 1 for project location on USGS Topographic Map.

1.3 Site Topography

The existing natural grass field is crowned with a one and a half to two (1.5-2.0) percent pitch, draining east and west towards the track (center of field elevation: 105.75). To the north of the

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track and field, the topography slopes upwards approximately six (6) feet to the tennis courts (elevation: 112.00). To the east of the track and field the grade pitches down into a swale and then upwards three feet to an asphalt walkway/access drive (elevation: 106.00). To the south and west, the topography slopes down approximately three to four feet into the woodlands and wetland pockets (elevation 100.00).

1.4 Receiving Waters

The Saint Joseph's campus generally drains from the northeast to southwest. The developed area is surrounded by several wetlands that drain towards the southwest end of campus to more wetlands. The runoff eventually drains to Trout Brook which is part of the Subregional Basin 4403. The Town of West Hartford is included in the "Connecticut" major watershed area.

The analysis described in this report focuses on the area disturbed by the athletic improvements and will demonstrate that the peak runoff rates to the wetlands are reduced under all design storm conditions due to the installation of the stone base underneath the synthetic turf field and the removal of impervious area (track surfacing and sidewalk areas).

1.5 Soils Conditions

The Natural Resources Conservation Service (NRCS) Web Soil Survey identifies the following predominant soil type within the disturbed area:

• Udorthents Smoothed— This designation is reflective of disturbed conditions where few, if any remnants exist of the natural soil horizons. Erosion factors for use in the Universal Soil Loss Equation are K=0.28, and T=3. Udorthents soils are classified as Hydrologic Soil Group C.

Geotechnical investigations indicate that the site is at the interface of moraine deposits (medium compact sand, silt, and gravel) and glacial lake deposits (stiff silt and clay). The water table is within five (5) feet of finish grade, with all soils three (3) feet below grade being fully saturated from capillary water. The complete subsurface exploration program and geotechnical engineering evaluation report by Welti Geotechnical, P.C. is included as part of this application for reference.

1.6 Historic Flooding

The project area is not identified within a flood area on the FEMA Firm Map No. 09003C0361F effective 09/26/2008. A copy of the FEMA map is included within this report.

1.7 Alterations to Natural Drainageways

The new project area drains to an existing piped system. The proposed project replicates these drainageways.

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1.8 Methodology and Modeling Assumptions

Runoff and routing calculations have been performed for the watershed areas impacted by the project in both the pre-development and post-development conditions using HydroCAD© software. Time of concentration and runoff curve number calculations have been determined using the method described in NRCS Technical Release 55 — Urban Hydrology for Small Watersheds (TR-55). Time of concentration calculations have been amended where the value given by the TR-55 method is less than five minutes. In these cases a standard minimum value of five minutes has been used to keep this parameter within the acceptable working range of the model.

Design rainfall events have been modeled using the SCS Type III hydrograph for 24-hour duration storms. The rainfall depth for each return period is taken from *Table 7-2 – 2004 Connecticut Stormwater Quality Manual and ConnDOT Drainage Manual (2004)*. The rainfall depth values for standard design storm frequencies are given in the table below.

24-Hour Rainfall Dep	oths for Hartfor	d County, Conn	ecticut at Design	n Storm Freque	encies
Table 7-2 – 2004 Co.	nnecticut Storm	water Quality N	Лanual		
Frequency	1-Year	2-Year	10-Year	25-Year	100-Year
Rainfall Depth(in)	2.6	3.2	4.7	5.5	6.9

2 STORMWATER ANALYSIS

2.1 Pre-Development Conditions

The pre-development condition has been analyzed at one design point. Design Point 1 (DP-1) is a catch basin at the wetland to the south of the project site. This catch basin has a 36" inlet and 36" outlet which is directed towards the southwestern end of the campus. The stormwater eventually enters Trout Brook to the southwest of campus.

The pre-development conditions analysis has been broken out into four (4) subcatchment areas:

- SC-A includes the natural grass field and a majority of the track running lanes and runways. Stormwater from this area drains to the collector pipes along the east and west edge of the field and connect into the large catch basin in the south D-zone. This large catch basin outlets via 36" RCP to the catch basin in the south wetland.
- SC-B includes the south D-zone area, the perimeter lawn area, and the wetlands to the south. Stormwater flows overland to the catch basin in the south wetland.
- SC-C includes the existing sidewalks and lawn area to the east of the running track. This area drains to several yard drains that connect to the large catch basin in the south Dzone and outlets via 36" RCP to the catch basin in the south wetland.
- SC-D includes the lawn area to the north of the track. Stormwater is collected by a series of yard drains, which connect to the field header pipe. The header pipe connects to the large catch basin in the south D-zone and outlets via 36" RCP to the catch basin in the south wetland.

Detailed descriptions of the subcatchment areas can be found in the HydroCAD runoff reports and on the pre-development watershed plan.

2.2 Post-Development Conditions

The same total drainage area and design point were analyzed in the post-development condition. The major changes in coverage are the construction of a new synthetic turf field and the removal of the existing running track and adjacent sidewalk. Approximately 30,500 sf of impervious area is being removed as a result of the proposed project.

The synthetic turf field is included in the model as Direct Entry (CN 98) since there is no depression storage, or evapotranspiration loss of rainfall that lands on the structure. Rainfall will drain directly through the surface of the field to the underlying base layer of highly porous crushed stone. The stone base will act as a large storage reservoir, detaining rainfall that enters the structure. It should be noted that the stone layer extends 6 inches beneath the field underdrain piping, providing significant storage volume prior to *any* stormwater discharging to the piped drainage system. The stone base layer is proposed to be the reclaimed/recycled track base stone material. If there is an inadequate volume of reclaimed materials for re-use, borrow crushed stone will be installed. This material is modeled as a pond with 30% voids.

Based on the geotechnical investigation, the soils are very saturated and will not infiltrate into the subsoils underneath the field. Therefore, no exfiltration has been used in the HydroCAD model.

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The underdrains are modeled as multiple vertical orifices that discharge to the larger collector pipes that collect and convey stormwater around the perimeter of the proposed turf field. The header pipe system will convey the rainfall to the existing structure to the south of the field.

Similar to the pre-development analysis, the post-development conditions analysis has been broken out into four (4) subcatchment areas:

- SC-A includes the synthetic turf field. Stormwater from this area will drain vertically to the panel drains which connect into the collector pipes along the east and west edge of the field. The collector pipe connects into the large catch basin in the south D-zone. This large catch basin outlets via 36" RCP to the catch basin in the south wetland.
- SC-B includes the south D-zone area that is to remain and the perimeter lawn area and wetlands to the south. Stormwater flows overland to the catch basin in the south wetland.
- SC-C includes the existing sidewalks and lawn area to remain to the east of the synthetic turf field. This area drains to several yard drains that connect to the large catch basin in the south D-zone and outlets via 36" RCP to the catch basin in the south wetland.
- SC-D includes the lawn area to the north of the track. Stormwater is collected by a series of yard drains, which connect to the field header pipe. The header pipe connects to the large catch basin in the south D-zone and outlets via 36" RCP to the catch basin in the south wetland.

Detailed descriptions of the subcatchment areas can be found in the HydroCAD runoff reports and on the post-development watershed plan. The runoff and routing analysis shows that there will be no increase in peak runoff from the proposed development under any design storm conditions.

Refer to the tables below for the peak flow and runoff volume comparisons in the 1-year, 2-year, 10-year, 25-year and 100-year storms.

Ta	Table 1 - Development Runoff Summary- Peak Flow (cfs)												
Docigo Point 1	Design Storm Event Return Period												
Design Point-1	1-Year	2-Year	10-Year	25-Year	100-Year								
Pre-Dev	7.39	10.52	18.76	23.26	31.17								
Post-Dev	4.20	6.03	11.48	15.31	21.58								
Change	-3.19	-4.49	-7.28	-7.95	-9.59								

Ta	Table 2 - Development Runoff Summary- Volume (acre-ft)												
Design Point-1	Design Storm Event Return Period												
Design Point-1	1-Year 2-Year 10-Year 25-Year 100-Year												
Pre-Dev	0.558	0.788	1.409	1.757	2.381								
Post-Dev	0.422	0.672	1.326	1.686	2.325								
Change	-0.136	-0.116	-0.08	-0.071	-0.056								

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2.3 Best Management Practices (BMPs) / State of Connecticut Requirements

No specific stormwater BMPs are proposed to be constructed as part of this project because there is no increase in impervious area. The majority of the running track and adjacent sidewalk are proposed to be removed, which results in a reduction of approximately 30,500 sf of impervious area from the pre- to post-development condition.

State Requirements:

- Section 7.4 Pollutant Reduction
 - o There is no added impervious area to the proposed project; therefore, a specific BMP for the treatment of the water quality volume is not required.
 - o Installation of the synthetic turf field eliminates the need for fertilizers and other legal lawn treatments; therefore, reducing the amount of these materials which can have a detrimental effect on the wetland habitat.
 - o The field project proposes no vehicular use, so oil spills and other hazardous materials typical of parking lots/driveways will not be an issue.
- Section 7.5 Groundwater Recharge
 - O Due to the highly saturated soils and high groundwater level, provisions to address the groundwater recharge volume are not feasible.
- Section 7.6 Peak Flow Control
 - o Stream Channel Protection
 - The 2-year, 24-hour post-development peak flow rate will be reduced to less than the 1-year, 24-hour pre-development peak flow rate.
 - 1-year pre-development flow rate = 7.39 cfs
 - 2-year post-development flow rate = 6.03 cfs
 - Conveyance Protection
 - The project's system is designed to the 10-year, 24-hour storm.
 - o Peak Runoff Attenuation
 - The post-development peak flows will not result in any significant increase in the peak runoff from the site during design storm events of 2-year, 10-year, 25-year, and 100-year return periods.
 - Emergency Outlet Sizing
 - There are no proposed changes to the downstream outlets and the 100year peak flow rates are being reduced; therefore, there should be no erosion at the existing outlets.

3 CONCLUSIONS

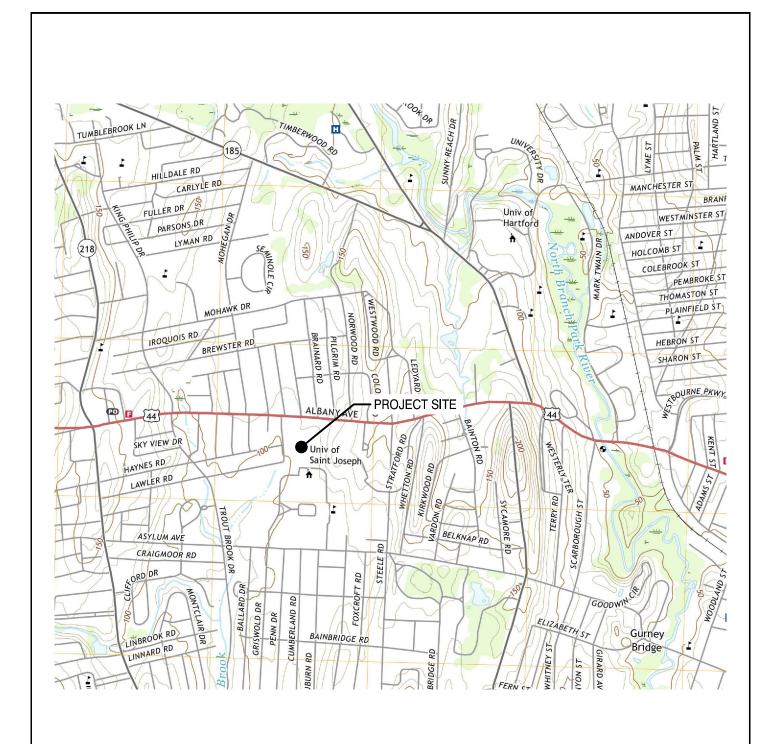
The runoff and routing calculations demonstrate that the development will not result in any increase in the peak runoff from the site during design storm events of the 1-year, 2-year, 10-year, 25-year and 100-year return periods. Due to the installation of the large porous stone reservoir underneath the field, the project will be able to store runoff beneath the field before outletting from the system. Also, with the installation of synthetic turf in lieu of natural grass, the project will eliminate the need for fertilizers and other legal lawn treatments. Therefore, the project will not result in any adverse impact on the downstream wetlands or watershed.

4 REFERENCES

- CT Stormwater Manual (2004 and as amended)
- Connecticut Guidelines for Soil Erosion and Sediment Control (2002)
- NRCS Technical Release 378
- NRCS Web Soil Survey
- Geotechnical Study for Synthetic Turf Field at University of Saint Joseph, 1678 Asylum
 Avenue, West Hartford, CT by Welti Geotechnical, P.C. dated January 25, 2019

University of Saint Joseph – Athletic Field Renovations Stormwater Management Report

Figures





ARCHITECTURE ENGINEERING PLANNING INTERIOR DESIGN **COMMISSIONING**

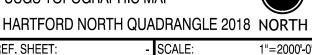
University of Saint Joseph Athletic Field Renovations

West Hartford, Connecticut

200 Brickstone Square, Ste. 303 Andover, MA 01810 tel. (978) 474-1721 fax. (978) 474-1742 www.smrtinc.com

SUBJECT:

USGS TOPOGRAPHIC MAP



SKETCH No.



REF. SHEET		-
PROJECT M		RFW
A/E OF REC		MAF
CAD FILE:	19014-TC	OPO MAP
PROJECT N	0:	19014
DATE:		2-20-19

FIG. 1 COPYRIGHT 2018 SMRT INC

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Soils Information Appendix A

CONTENTS:

- 1. NRCS Hydrologic Soil Group Map
- 2. Geotechnical Study for Synthetic Turf Field at University of Saint Joseph, 1678 Asylum Avenue, West Hartford, CT by Welti Geotechnical, P.C. dated January 25, 2019



MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:12.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D **Soil Rating Polygons** Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D contrasting soils that could have been shown at a more detailed Streams and Canals В Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. B/D Soil Survey Area: State of Connecticut Survey Area Data: Version 18, Dec 6, 2018 C/D Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. D Date(s) aerial images were photographed: Aug 27, 2016—Oct Not rated or not available 30. 2017 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
9	Scitico, Shaker, and Maybid soils	C/D	1.6	5.1%
25A	Brancroft silt loam, 0 to 3 percent slopes	С	4.5	13.9%
40B	Ludlow silt loam, 3 to 8 percent slopes	С	4.5	13.7%
225B	Brancroft-Urban land complex, 0 to 8 percent slopes	С	6.9	21.2%
306	Udorthents-Urban land complex	В	8.2	25.3%
308	Udorthents, smoothed	С	6.8	20.8%
Totals for Area of Inter	rest	1	32.6	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

WELTI GEOTECHNICAL, P.C.

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January 25, 2019

Mr. Richard F. Webb, Senior Principal SMRT One Dundee Park, Suite 4 Andover, MA 01810

Re: Geotechnical Study for Synthetic Turf Field at University at University of Saint Joseph, 1678 Asylum Avenue, West Hartford, CT

Dear Dick:

- **1.0** Herewith are the data from the test borings taken at the above site. Five borings were drilled to a depth of five feet at the existing field. The borings were sampled continuously. The boring locations are shown on the attached plan. The borings were drilled and sampling was conducted by this firm solely to obtain indications of subsurface conditions as part of a geotechnical exploration program. No services were performed to evaluate subsurface environmental conditions. Grain size gradation and water content tests were performed on four soil samples taken from the field borings and two laboratory permeability test were performed on two representative soil samples. The results of the laboratory tests are included with the borings logs.
- **2.0** The **Subject Project** is a proposed new synthetic turf surface over the existing natural turf field. The existing running track will be removed and the new field may be moved closer to the tennis courts. A site layout and grading plan was not available at the time of this report.
- **3.0** Geologically the site is at the interface of the moraine deposits and the glacial lake deposits. The moraine deposits consist medium compact sandy silt with little gravel. The lake deposits consist primarily of medium stiff silt with little clay. There are shallow fills atop the natural deposits.
- **3.1** The **Soils Cross Sections** from the test borings is generally as follows:

Topsoil to 8" to 12"

Locally; fine SAND, little to some Silt to 1.5 to 2 feet, medium compact

SILT, trace to little fine Sand and Clay, trace Gravel to 5+ feet, medium stiff/medium compact

- **3.2** The **Water Table** was not evident on boring completion, but water contents of samples below 3 feet are at or close to saturation. It should be assumed that the water table will within 5 feet of grade and that the soil 2 feet above the water table would be saturated from capillary water.
- **4.0** The general criteria for the layers directly under synthetic fields are usually part of a design build section. Apart from this section the sub grades must be capable of supporting the construction equipment without rutting. A second requirement would be a total section (including synthetic field section) of least 18" of non-frost susceptible soils. Based on the grain size gradation tests and inspection of the soil samples, the soils at the subgrade below the topsoil have silt contents ranging from 30 to 80%. These soils would not be considered as non frost susceptible.
- **4.1** Controlled fills for frost protection should conform to the following or be with 3/4" crushed stone (CTDOT Form 817, M.01.01, No.6 Stone):

Percent Passing	Sieve Size
100	1.5"
90 - 100	1"
75 - 100	3/4"
10 - 35	1/4"
3 - 12	No. 100
0 - 5	No 200

- **4.2** The **initial step at the subject site** would be stripping the topsoil. The excavations should be made with a small bulldozer or excavator with smooth bucket to minimize disturbance to the subgrade soils. If the sub grades can be proof rolled without significant rutting or movements, the granular base would be placed over the stripped subgrade and compacted to 95+ % of modified optimum density (ASTM 1557D). The removal of the track may provide some granular material for part of the 18" of fill. If the sub grades are wet or unstable, proof compaction should be avoided or halted, if already initiated. The initial layer of material should be with a minimum 12" layer of 3/4" (No. 6 Stone) placed over a stabilization fabric (Mirafi HP 570 or equal). The working surface above the crushed stone should conform to the gradation in section 4.1. The total depth of crushed stone and controlled fill to provide a stable subgrade for heavy equipment would be at least 18". No compaction is required on the stone layer. The controlled fill should be rolled initially with 1 or 2 passes of a vibratory roller. If there is any weaving, the compaction should be solely with static rolling.
- **5.0** This report has been prepared for specific application to the subject project in accordance with generally accepted soil and foundation engineering practices. No other warranty, express or implied,

is made. In the event that any changes in the nature, design and location of structures are planned, the conclusions and recommendations contained in this report should not be considered valid unless the changes are reviewed and conclusions of this report modified or verified in writing.

The analyses and recommendations submitted in this report are based in part upon data obtained from referenced explorations. The extent of variations between explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

Welti Geotechnical, P.C., should perform a general review of the final design and specifications in order that geotechnical design recommendations may be properly interpreted and implemented as they were intended.

If you have any questions please call me.

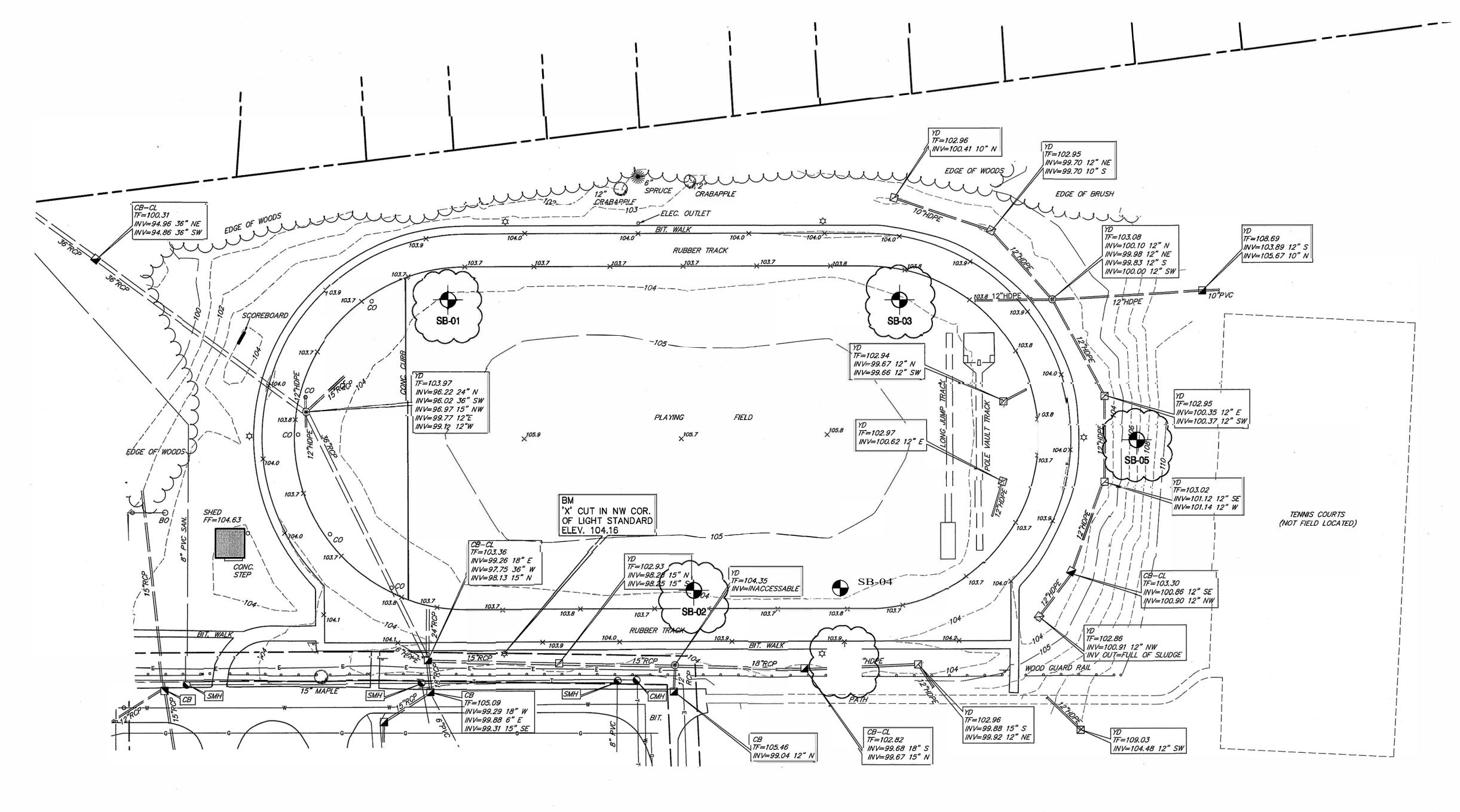
Very truly yours,

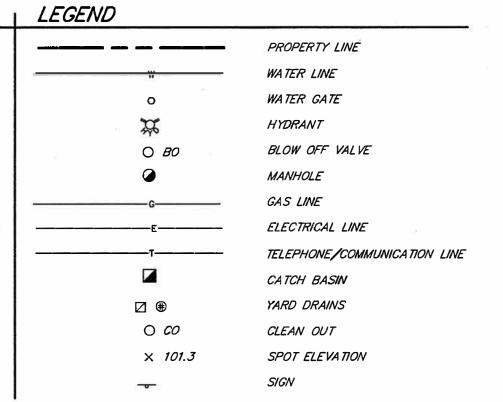
Max Welti, P.E.

President, Welti Geotechnical, P.C.

Clarence Welti, PhD, P.E.

Vice President





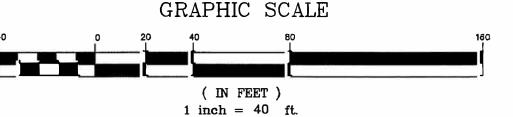
THIS DRAWING HAS BEEN PREPARED BASED, IN PART, ON INFORMATION PROVIDED BY OTHERS RELATING TO THE LOCATION OF UNDERGROUND SERVICES. WE CAN NOT VERIFY THE ACCURACY OF THIS INFORMATION AND SHALL NOT BE HELD RESPONSIBLE FOR ANY ERRORS OR OMISSIONS WHICH MAY BE INCORPORATED HEREIN AS A RESULT. INDICATED UNDERGROUND UTILITIES ARE BASED ON AVAILABLE DATA. THE LOCATIONS ARE APPROXIMATE AND ALL UTILITIES MAY NOT BE SHOWN.

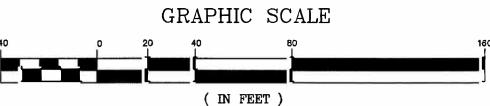
CALL "CALL B-4-U DIG" AT 1-800-922-4455 PRIOR TO ANY EXCAVATION.

University of

SAINT JOSEPH

CONNECTICUT





SITE BORING PLAN CLARENCE WELTI ASSOCIATES, INC. 1/15/19



"TOPOGRAPHIC SURVEY, PLAN PREPARED FOR SAINT JOSEPH COLLEGE, 1678 ASYLUM AVENUE, WEST HARTFORD, CONN." DATE: 10-19-2007, SCALE: 1"=40', BY MEEHAN &

NOTES:

1. THIS SURVEY HAS BEEN PREPARED PURSUANT TO THE REGULATIONS OF CONNECTICUT STATE AGENCIES SECTIONS 20-300b-1 THROUGH 20-300b-20 AND THE "STANDARDS FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT" AS ADOPTED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INC. ON SEPTEMBER 26, 1996;

TYPE OF SURVEY: IMPROVEMENT LOCATION SURVEY

CLASS OF HORIZONTAL ACCURACY: A-2 CLASS OF VERTICAL ACCURACY: V-2 CLASS OF TOPOGRAPHIC ACCURACY: T-2 SURFACE UTILITY ACCURACY: A-2 UNDERGROUND UTILITY ACCURACY: D (COMPILED)

2. THIS MAP IS VALID ONLY IF IT BEARS THE ORIGINAL SIGNATURE AND EMBOSSED SEAL OF THE UNDERSIGNED LAND SURVEYOR.

TO MY KNOWLEDGE AND BELIEF, THIS MAP IS SUBSTANTIALLY CORRECT AS

SURVEYOR'S SIGNATURE LICENSE NUMBER

07172-TRACK2.DWG

01-06-18

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N A

OF

APROVEMEN-LOCATION SURVEY

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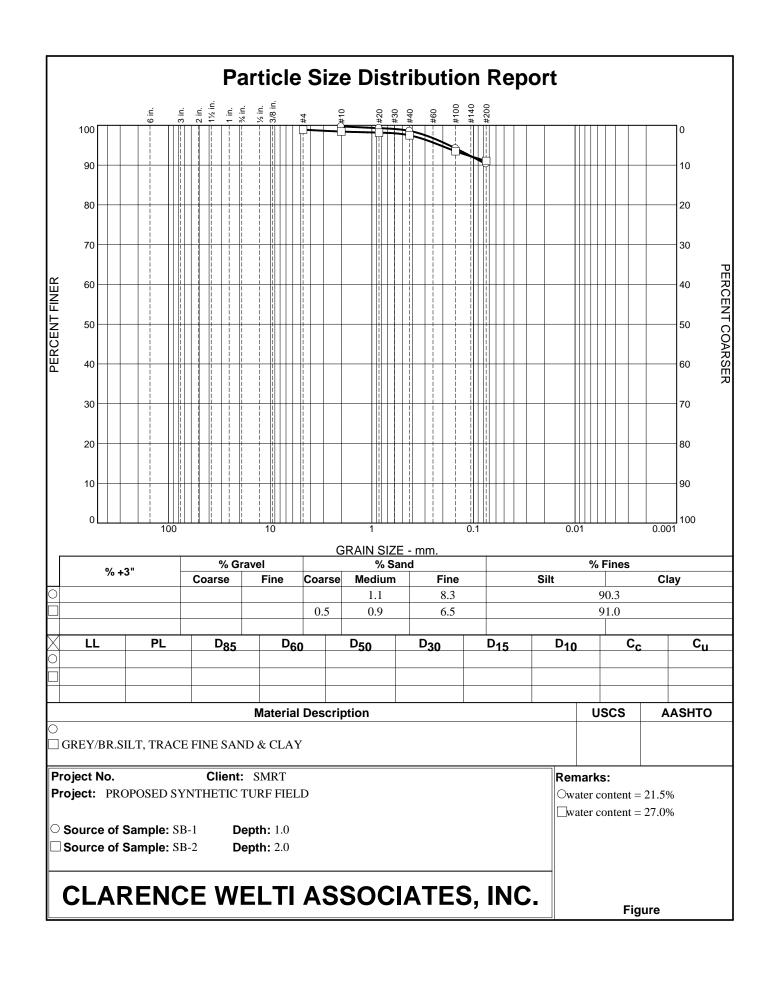
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				30"				E. COORDINATE	AT	FT. AFTER	HOURS	FINISH DATE	1/1	5/19
HAMME	K FALL			30	$-\perp$									
DEPTH	NO.	SAM		DTH	Α			STRATUM	DESCRIF + REM					ELEV.
0		BLOWS/6"					ΤO	PSOIL	+ KEWI	AKKS		-	\dashv	
	1	3-3-3-5	0.0-	-2.0'				FINE SAND, LITTLE SILT					1.0	
						_:::::::	BR	. SILT, TRACE CLAY					1.0	
	2	3-4-5-6	2.0'-	-4.0'		_::::::							3.0	
							RE	D/BR.SILT, TRACE FINE SAI	ND & GR	AVEL		\	0.0	
_ [3	4-8	4.0'-	-5.0'		T::::::							_	
5						1	ВО	TTOM OF BORING @ 5.0'					5.0	
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									DBILLE	R: K. CHRIST	ΙΔΝΙΛ			
LEGEN	ND: COI	A:							INSPECT		IAINA			
SAMPI	E: D=DRY A=	-AUGER C=0	CORE U	=UNDIS	TURBED	PIST	ON S=SPLIT SPOON	TABLEC.	- T					
PROPO	ORTION	S USED: TRA	ACE=0-10% I	LITTLE=	10-20%	SOME=20	0-35%	6 AND=35-50%	SHEET	1 OF 1	HOLE NO).	SB	-1

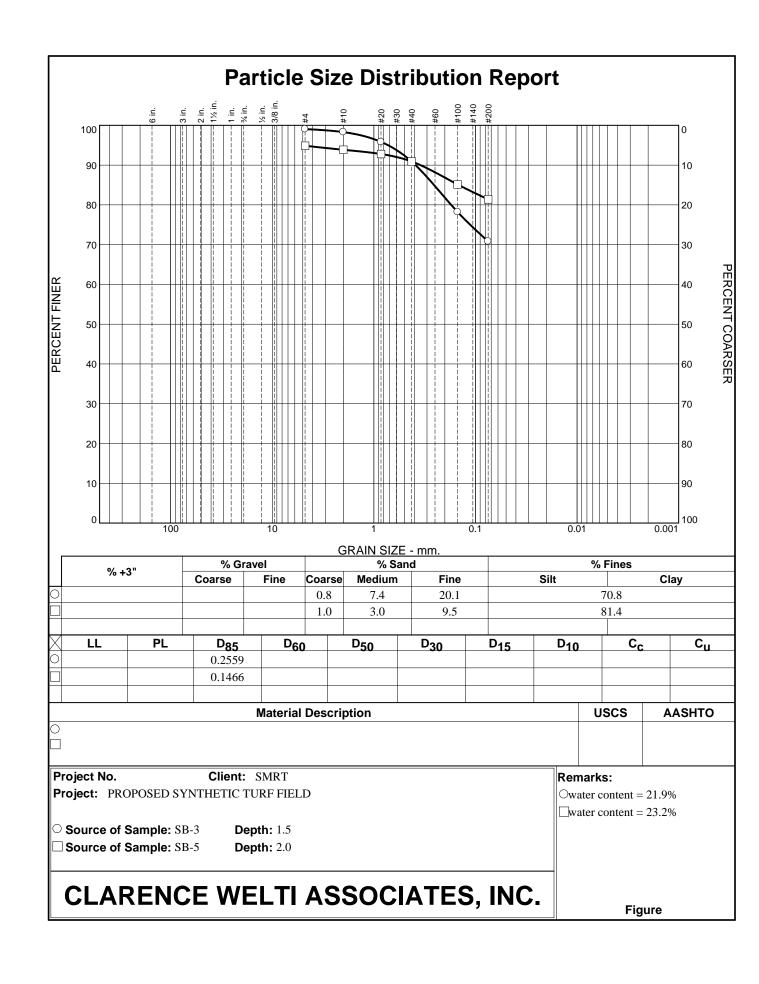
CLA	DENC	E WELTLA	V660C I	INIC	CLIEN	T			PROJE	CT NAME				
	BOX 39	E WELTI A	155UC., I	INC.					PROPOSED SYNTHETIC TURF FIELD LOCATION UNIVERSITY OF SAINT JOSEPH 1678 ASYLLIM AVENUE WEST HARTFORD CT					
		JRY, CONN	06033						LOCA	TION UNIVERSI	TY OF SA	OL TAI	SEPH	
		JICT, OOM						SMRT	1070	7 TO I LOW 7 TV	<u>EŅUE, W</u>	EST HAI	RTF	DRD, CT
		AUGER	CASING	SAMPI	LER	CORE BA	AR.	OFFSET	SURFAC	E ELEV.	HOLE	NO.	SI	3-2
TYPE		HSA		SS	;			LINE & STA.	GROUI	ND WATER OBSEF	DVATIONS	START		
SIZE I.D		3.75"		1.37	5"		-	N. COORDINATE		NE FT. AFTER C		DATE	1/1	5/19
HAMME	R WT.			1401	bs				AT	FT. AFTER	HOURS	FINISH		
HAMME	R FALL			30'				E. COORDINATE	AI	FI. AFIEK	HOURS	FINISH DATE	1/1	5/19
		SAM	PLE					STRATUM	DESCRIE	PTION				
DEPTH	NO.	BLOWS/6"		PTH	Α				+ REM.					ELEV.
0	1	3-4-6-8 0.0'-2.0' TO						PSOIL						
Ī							BR.	FINE-MED.SAND, SOME SIL	_T, TRAC	CE GRAVEL			1.0	
	2	5-7-8-8	2 0'-	-4.0'			GRI	EY/BR.SILT, TRACE FINE SA	AND & C	LAY			2.0	
ŀ	-	0.00	2.0	1.0		-								
-	3		4.01	F 0'		-								
5	3	6-6	4.0	-5.0'			BO.	TTOM OF BORING @ 5.0'					5.0	
						4	ВО	TOWOF BOKING @ 5.0						
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LEGEN	ND: COI	. A:								R: K. CHRIST	IANA			
SAMPI	E: D=DRY A=	AUGER C=0	CORE U	=UNDIS	TURBED) PISTO	ON S=SPLIT SPOON	INSPECT	TOR:					
								AND 25 500/	SHEET	1 OF 1	HOLE NO		SB	-2
									SILLEI	1 01 1	11022 140		30	

CLA	RENC	E WELTI A	ASSOC I	NC.	CLIEN	T		PROJECT NAME			
P.O.	BOX 39	7						PROPOSED LOCATION	SYNTHET	IC TURF	FIELD
GLAS	STONBL	JRY, CONN	06033				SMRT	LOCATION UNIVERS 1678 ASYLUM AV	ITY OF SA /FNUF. WI	INT JOSE FST HAR	PH TFORD, CT
		AUGER	CASING SAMPLER CORE BAR.				OFFGER	SURFACE ELEV.	HOLE		SB-3
TYPE		HSA		SS	;		LINE & STA.	GROUND WATER OBSE	PVATIONS	GT + DT	
SIZE I.D	-	3.75"		1.37	5"		N. COORDINATE	AT NONE FT. AFTER		DATE	1/15/19
HAMME	R WT.			1401	bs			AT FT. AFTER	HOURS	FINISH	1/15/19
HAMME	R FALL			30'	'		E. COORDINATE			FINISH . DATE	1/15/19
DEPTH		SAM			A		STRATUM		ELEV.		
0	NO.	BLOWS/6"		PTH		1	TOPSOIL	+ REMARKS			
	1	1-2-3-5	0.0	-2.0'		:::::::	BR.FINE-MED.SAND, LITTLE	SII T		1	.0
ŀ	2	4-6-6-7	2.0'	-4.0'		-	BR. SILT, SOME FINE-MED.S			1	.5
ŀ		4-0-0-7	2.0	-4.0			RED/BR.SILT, LITTLE FINE-M	FD.SAND		<u>3</u>	.0
ŀ	3	6-8	4.0'.	-5.0'		-	BR.SILT, TRACE FINE SAND			4	.0
5	-	0-0	7.0	3.0		:::::::	BOTTOM OF BORING @ 5.0'				.0
-						1					
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	ND: COL		=AUGER C=	CORE U	=UNDIS	TURBED	PISTON S=SPLIT SPOON	DRILLER: K. CHRIS' INSPECTOR:	TIANA		
							0-35% AND=35-50%	SHEET 1 OF 1	HOLE NO). [SB-3

C1 A	DENC	E WELTL	V660C	INIC	CLIEN	lТ			PROJECT NAME				
	BOX 39	E WELTI A	45506.,	INC.					PROPOSED S	YNTHET	IC TURE	FIE	LD
		, JRY, CONN	06033						LOCATION UNIVERSIT	Y OF SA	SOL TAI	SEPH	
02/		JICT, OOTHIT			<u> </u>			SMRT	1678 ASYLUM AVE	<u>ŅUE, WI</u>	EST HAI	RTF	DRD, CT
		AUGER	CASING	SAMP	LER	CORE B.	BAR.	OFFSET	SURFACE ELEV.	HOLE	NO.	SI	3-4
TYPE		HSA		SS	;		ī	LINE & STA.	GROUND WATER OBSERV	VATIONE	START		
SIZE I.D		3.75"		1.37	5"			N. COODDINATE	AT NONE FT. AFTER 0		DATE	1/1	5/19
HAMME				1401				N. COORDINATE					
				30'			I	E. COORDINATE	AT FT. AFTER	HOURS	FINISH DATE	1/1	5/19
HAMME	EK FALL			30							<u></u>		
DEPTH	NO	SAM		DTH	A			STRATUM	DESCRIPTION + REMARKS				ELEV.
0	NO.	BLOWS/6"		PTH		+	TOF	PSOIL	+ KEWIAKKS			\dashv	
	1	2-3-4-5	0.0	-2.0'		_		SILT, LITTLE FINE SAND, T	RACE CLAY & GRAVEL	L		0.66	
						_:::::::	•	EY/BR.SILT, TRACE FINE S				1.5	
	2	4-3-3-6	2.0'	-4.0'				,	•				
							:						
	3	4-3	4 0'	-5.0'		- :::::::	:						
5 –			1.0	0.0			BOT	TOM OF BORING @ 5.0'				5.0	
-						-		Tom or Bornito 9 0.0					
10						\dashv							
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15 –						-							
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35								Т	DDILLED IV OUDICE:	A N I A			
LEGE	ND: COL	A:							DRILLER: K. CHRISTIA	ANA			
SAMPI	LE TYPI	E: D=DRY A	=AUGER C=	CORE U	=UNDIS	TURBED	D PISTO	ON S=SPLIT SPOON	INSPECTOR:				
PROPO	ORTION	S USED: TR.	ACE=0-10%	LITTLE=	10-20%	SOME=2	20-35%	AND=35-50%	SHEET 1 OF 1 I	HOLE NO)	SB	-4
	- '	•							STIPPI OL I	TOLL INC	•	J D	7

OL 4	DENO	_ \A/_! T! /	10000	o	CLIE	NT			PROJE	ECT NAME				
	BOX 39	E WELTI A	4550C., I	INC.						PROPOSED :	SYNTHET	IC TURF	FIE	LD
		JRY, CONN	06033						LOCA	TION UNIVERSI	TY OF SA	INT JOS	FPH	
OLA	310110	JICT, CONT						SMRT	1678	3 ASYLUM AV				
		AUGER	CASING	SAMP	LER	CORE BA	AR.	OFFSET	SURFAC	E ELEV.	HOLE	NO.	SE	3-5
TYPE		HSA		SS	;			LINE & STA.	CDOLL	ND WATER OBSE	N/A TIONG	CTADT		
SIZE I.D	١.	3.75"		1.37	5"			N COODDINATE		ND WATER OBSER N e ft. after (START DATE	1/1	5/19
HAMME	ER WT			1401				N. COORDINATE						
HAMME				30'				E. COORDINATE	AT	FT. AFTER	HOURS	FINISH DATE	1/1	5/19
112 (1011/11)	EKTALE	SAM	DI E					CTD ATLIM	DECCRI	OTION			T	
DEPTH	NO.	BLOWS/6"		PTH	A			STRATUM	+ REM					ELEV.
0	1	2-1-3-4	_	-2.0'		:::::::	∖TOI	PSOIL				0	.25	
							GR	EY/BR.SILT, LITTLE FINE SA	AND, TR	ACE CLAY &	GRAVEL			
	2	4-4-5-5	2.0'	-4.0'										
		4-4-0-0	2.0	-4.0										
	_					- ::::::								
5 –	3	4-4	4.0'	-5.0'				TTOM OF BODING O F O					5.0	
						_	BO	TTOM OF BORING @ 5.0'				`		
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						1								
35 _						1								
									DRILLE	R: K. CHRIST	IANA			
	ND: COI			G075 -			Dre-	ON G GDI IT GDO OT	INSPECT		47 1			
	SAMPLE TYPE: D=DRY A=AUGER C=CORE U=UNDISTUE PROPORTIONS USED: TRACE=0-10% LITTLE=10-20% SOI							Г						_
PKUP	JKIIUN	S USED: TRA	ACE=U-1U%	LIIILE=	10-20%	SUME=2	.0-33%	0 AND=55-50%	SHEET	1 OF 1	HOLE NO).	SB	-5





Proposed Synthetic Surface Athletic Field University of Saint Joseph 1678 Asylum Avenue, West Hartford, CT

Laboratory Permeability Test

Sample No.	Depth (feet/day)
SB-1, 2'-4'	<1.0
SB-3, 2'-4'	<1.0

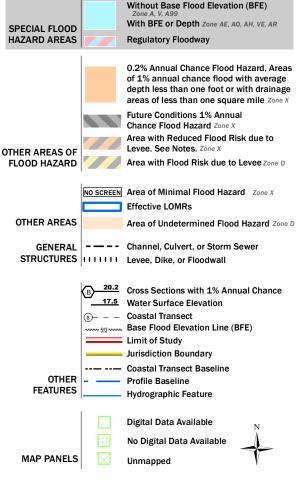
University of Saint Joseph – Athletic Field Renovations Stormwater Management Report FEMA Flood Insurance Rate Map Appendix B

National Flood Hazard Layer FIRMette



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT



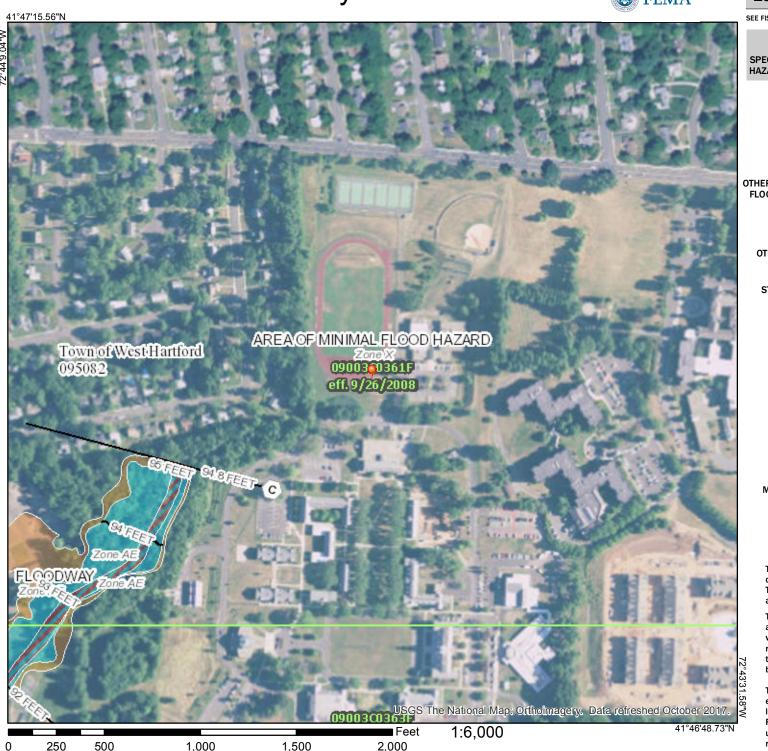
•

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 2/11/2019 at 3:22:54 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



University of Saint Joseph – Athletic Field Renovations

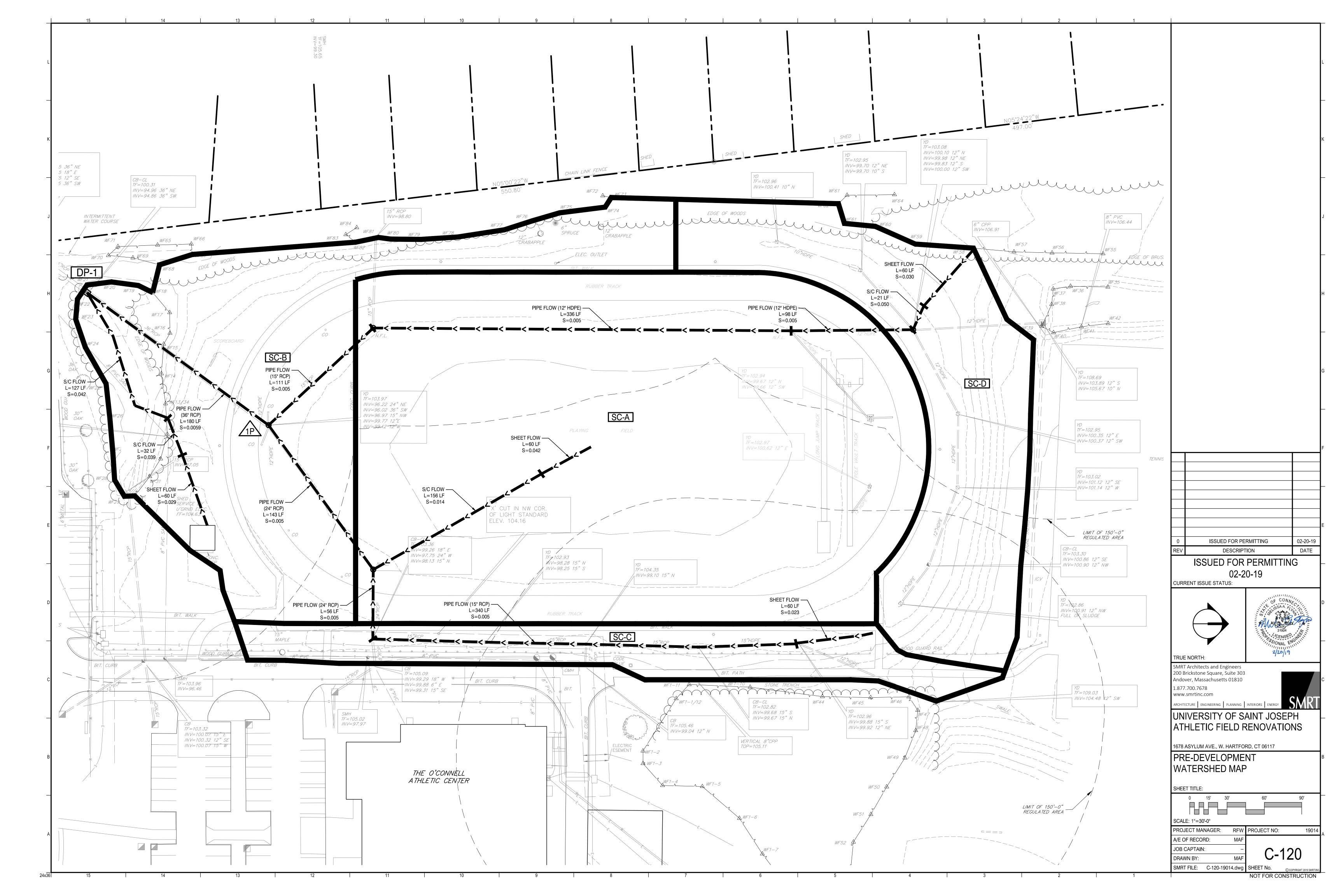
Stormwater Management Report

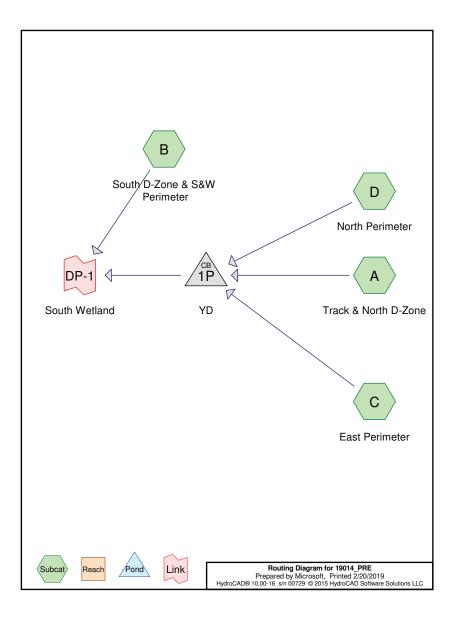
Pre-Development Conditions Analysis

Appendix C

CONTENTS:

- 1. Pre-development Watershed Map
- 2. Pre-development HydroCAD Runoff and Routing Calculations





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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
4.357	79	50-75% Grass cover, Fair, HSG C (A, B, C, D)
0.254	98	Asphalt Pavement (B, C, D)
0.009	98	Roof (B)
1.104	98	Running Track (A, B)
5.725	84	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
 (acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
4.357	HSG C	A, B, C, D
0.000	HSG D	
1.367	Other	A, B, C, D
5.725		TOTAL AREA

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Ground Covers (all nodes)

 HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	4.357	0.000	0.000	4.357	50-75% Grass cover, Fair	A, B, C,
							D
0.000	0.000	0.000	0.000	0.254	0.254	Asphalt Pavement	B, C, D
0.000	0.000	0.000	0.000	0.009	0.009	Roof	В
0.000	0.000	0.000	0.000	1.104	1.104	Running Track	A, B
0.000	0.000	4.357	0.000	1.367	5.725	TOTAL AREA	

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Pipe Listing (all nodes)

Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Diam/Width	Height	Inside-Fill
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)
1	Α	0.00	0.00	143.0	0.0050	0.013	24.0	0.0	0.0
2	С	0.00	0.00	340.0	0.0050	0.013	15.0	0.0	0.0
3	С	0.00	0.00	56.0	0.0050	0.013	24.0	0.0	0.0
4	С	0.00	0.00	143.0	0.0050	0.013	24.0	0.0	0.0
5	D	0.00	0.00	98.0	0.0050	0.013	12.0	0.0	0.0
6	D	0.00	0.00	336.0	0.0050	0.013	12.0	0.0	0.0
7	D	0.00	0.00	336.0	0.0050	0.013	15.0	0.0	0.0
8	1P	96.02	94.96	180.0	0.0059	0.013	36.0	0.0	0.0

19014 PRE

Type III 24-hr 1-Year Storm Rainfall=2.60" Printed 2/20/2019

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Time span=0.00-40.00 hrs, dt=0.01 hrs, 4001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A: Track & North D-Zone

Runoff Area=122,732 sf 21.87% Impervious Runoff Depth=1.13"

Flow Length=359' Tc=6.7 min CN=83 Runoff=3.61 cfs 0.266 af

Subcatchment B: South D-Zone & S&W Runoff Area=62,857 sf 40.29% Impervious Runoff Depth=1.40" Flow Length=219' Tc=6.4 min CN=87 Runoff=2.33 cfs 0.168 af

Subcatchment C: East Perimeter

Runoff Area=21,545 sf 22.41% Impervious Runoff Depth=1.13"
Flow Length=599' Tc=8.4 min CN=83 Runoff=0.60 cfs 0.047 af

Subcatchment D: North Perimeter

Runoff Area=42,232 sf 6.09% Impervious Runoff Depth=0.96"
Flow Length=851' Tc=9.3 min CN=80 Runoff=0.94 cfs 0.077 af

1.01 25.1ga = 501 1.5=50 1.11 5.1=50 1.41

Pond 1P: YD Peak Elev=96.92' Inflow=5.08 cfs 0.390 af 36.0" Round Culvert n=0.013 L=180.0' S=0.0059 '/' Outflow=5.08 cfs 0.390 af

Link DP-1: South Wetland Inflow=7.39 cfs 0.558 af Primary=7.39 cfs 0.558 af Primary=7.39 cfs 0.558 af

Total Runoff Area = 5.725 ac Runoff Volume = 0.558 af 76.11% Pervious = 4.357 ac Average Runoff Depth = 1.17" 23.89% Impervious = 1.367 ac

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Type III 24-hr 1-Year Storm Rainfall=2.60"

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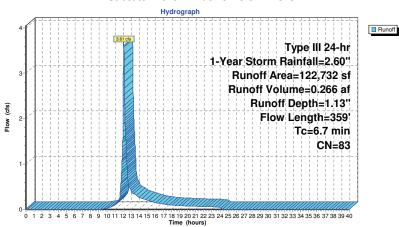
Summary for Subcatchment A: Track & North D-Zone

3.61 cfs @ 12.10 hrs, Volume= 0.266 af, Depth= 1.13" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Storm Rainfall=2.60"

_	Α	rea (sf)	CN E	escription						
*		26,841	98 F	98 Running Track						
		95,891	79 5							
	1	22,732	83 V	Veighted A	verage					
		95,891	7	8.13% Per	vious Area					
		26,841	2	1.87% Imp	ervious Ar	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	4.8	60	0.0420	0.21		Sheet Flow,				
						Grass: Short n= 0.150 P2= 3.20"				
	1.4	156	0.0140	1.90		Shallow Concentrated Flow,				
						Unpaved Kv= 16.1 fps				
	0.5	143	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP				
						24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'				
_						n= 0.013 Concrete pipe, bends & connections				
	6.7	359	Total							

Subcatchment A: Track & North D-Zone



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Type III 24-hr 1-Year Storm Rainfall=2.60" Printed 2/20/2019

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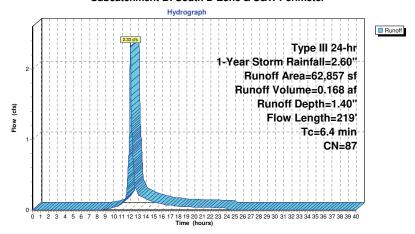
Summary for Subcatchment B: South D-Zone & S&W Perimeter

2.33 cfs @ 12.09 hrs, Volume= 0.168 af, Depth= 1.40" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Storm Rainfall=2.60"

	Д	rea (sf)	CN D	escription						
*		21,236	98 F	Running Track						
*		403		Roof						
*		3,684	98 A	sphalt Pav						
		37,534	79 5	0-75% Gra	ass cover, I	Fair, HSG C				
		62,857	87 V	Veighted A	verage					
		37,534	5	9.71% Per	vious Area					
		25,323	4	0.29% Imp	ervious Ar	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.6	60	0.0290	0.18		Sheet Flow,				
						Grass: Short n= 0.150 P2= 3.20"				
	0.2	32	0.0390	3.18		Shallow Concentrated Flow,				
						Unpaved Kv= 16.1 fps				
	0.6	127	0.0420	3.30		Shallow Concentrated Flow,				
_						Unpaved Kv= 16.1 fps				
	6.4	219	Total							

Subcatchment B: South D-Zone & S&W Perimeter



Type III 24-hr 1-Year Storm Rainfall=2.60"

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Summary for Subcatchment C: East Perimeter

0.60 cfs @ 12.12 hrs, Volume= 0.047 af, Depth= 1.13" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Storm Rainfall=2.60"

A	rea (sf)	CN D	escription		
*	4,828	98 A	sphalt Pav	/ement	
	16,717	79 5	0-75% Gra	ass cover, F	Fair, HSG C
	21,545	83 V	Veighted A	verage	
	16,717	7	7.59% Per	vious Area	
	4,828	2	2.41% Imp	ervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.2	60	0.0230	0.16		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
1.5	340	0.0050	3.72	4.57	Pipe Channel, Existing 15" RCP
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Concrete pipe, bends & connections
0.2	56	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
0.5	143	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
8.4	599	Total			

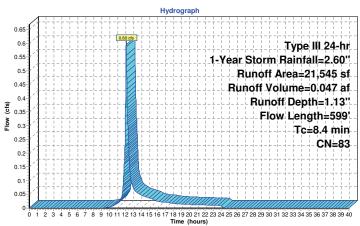
19014_PRE

Type III 24-hr 1-Year Storm Rainfall=2.60" Printed 2/20/2019

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Subcatchment C: East Perimeter





Type III 24-hr 1-Year Storm Rainfall=2.60"

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Summary for Subcatchment D: North Perimeter

0.94 cfs @ 12.14 hrs, Volume= 0.077 af, Depth= 0.96" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Storm Rainfall=2.60"

А	rea (sf)	CN E	escription		
*	2.573	98 A	sphalt Pav	/ement	
	39,659				Fair, HSG C
	42.232				un, ried e
	, -		Veighted A		
	39,659	-		vious Area	
	2,573	6	.09% Impe	ervious Are	a
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	in the second se
5.5	60	0.0300	0.18	```	Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
0.1	21	0.0500	3.60		Shallow Concentrated Flow,
					Unpayed Ky= 16.1 fps
0.5	98	0.0050	3.21	2.52	Pipe Channel, Existing 12" HDPE to Collector Pipe
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013 Corrugated PE, smooth interior
1.7	336	0.0050	3.21	2.52	
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013 Corrugated PE, smooth interior
1.5	336	0.0050	3.72	4.57	
1.0	000	0.0000	3.7L	1.07	15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Concrete pipe, bends & connections
	051	Takal			ii- 0.010 Concrete pipe, bends & connections
9.3	851	Total			

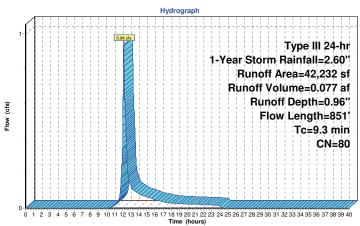
19014_PRE

Type III 24-hr 1-Year Storm Rainfall=2.60" Printed 2/20/2019

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Subcatchment D: North Perimeter





Type III 24-hr 1-Year Storm Rainfall=2.60"

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Summary for Pond 1P: YD

4.282 ac, 18.36% Impervious, Inflow Depth = 1.09" for 1-Year Storm event Inflow Area =

Inflow 5.08 cfs @ 12.11 hrs, Volume= 0.390 af

Outflow 5.08 cfs @ 12.11 hrs, Volume= 0.390 af, Atten= 0%, Lag= 0.0 min

Primary 5.08 cfs @ 12.11 hrs, Volume= 0.390 af

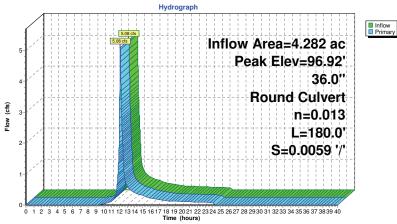
Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Peak Elev= 96.92' @ 12.11 hrs

Flood Elev= 103.97

Device	Routing	Invert	Outlet Devices
#1	Primary	96.02'	36.0" Round 36" RCP
			L= 180.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 96.02' / 94.96' S= 0.0059 '/' Cc= 0.900
			n= 0.013 Concrete nine, hends & connections, Flow Area = 7.07 sf

Primary OutFlow Max=5.07 cfs @ 12.11 hrs HW=96.92' (Free Discharge) 1=36" RCP (Barrel Controls 5.07 cfs @ 4.28 fps)

Pond 1P: YD



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Type III 24-hr 1-Year Storm Rainfall=2.60"

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Summary for Link DP-1: South Wetland

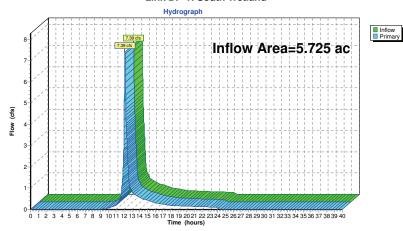
5.725 ac, 23.89% Impervious, Inflow Depth = 1.17" for 1-Year Storm event Inflow Area =

Inflow 7.39 cfs @ 12.10 hrs, Volume= 0.558 af

7.39 cfs @ 12.10 hrs, Volume= Primary = 0.558 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Link DP-1: South Wetland



Type III 24-hr 2-Year Storm Rainfall=3.20"

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Time span=0.00-40.00 hrs, dt=0.01 hrs, 4001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A: Track & North D-Zone	Runoff Area=122,732 sf	21.87% Impervious	Runoff Depth=1.61"
	Flow Length-359' Tc-6	7 min CN-83 Run	off_5 18 ofc 0 378 of

Subcatchment B: South D-Zone & S&W Runoff Area=62,857 sf 40.29% Impervious Runoff Depth=1.91" Flow Length=219' Tc=6.4 min CN=87 Runoff=3.19 cfs 0.230 af

Subcatchment C: East Perimeter Runoff Area=21,545 sf 22.41% Impervious Runoff Depth=1.61" Flow Length=599' Tc=8.4 min CN=83 Runoff=0.86 cfs 0.066 af

Subcatchment D: North Perimeter Runoff Area=42,232 sf 6.09% Impervious Runoff Depth=1.40" Flow Length=851' Tc=9.3 min CN=80 Runoff=1.41 cfs 0.113 af

Pond 1P: YD Peak Elev=97.11' Inflow=7.35 cfs 0.557 af 36.0" Round Culvert n=0.013 L=180.0' S=0.0059 '/' Outflow=7.35 cfs 0.557 af

Inflow=10.52 cfs 0.788 af Link DP-1: South Wetland Primary=10.52 cfs 0.788 af

> Total Runoff Area = 5.725 ac Runoff Volume = 0.788 af Average Runoff Depth = 1.65" 76.11% Pervious = 4.357 ac 23.89% Impervious = 1.367 ac

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Type III 24-hr 2-Year Storm Rainfall=3.20" Printed 2/20/2019

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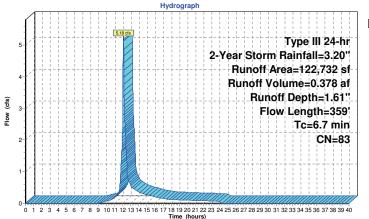
Summary for Subcatchment A: Track & North D-Zone

Runoff 5.18 cfs @ 12.10 hrs, Volume= 0.378 af, Depth= 1.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Storm Rainfall=3.20"

	A	rea (sf)	CN D	escription		
*		26,841	98 F	lunning Tra	ack	
		95,891	79 5	0-75% Gra	ass cover, F	Fair, HSG C
_	1	22,732	83 V	Veighted A	verage	
		95.891			vious Area	
		26.841	2	1.87% Imr	pervious Ar	ea
		,	_			
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
_	4.8	60	0.0420	0.21		Sheet Flow.
						Grass: Short n= 0.150 P2= 3.20"
	1.4	156	0.0140	1.90		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	0.5	143	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
						24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
						n= 0.013 Concrete pipe, bends & connections
_	6.7	359	Total			

Subcatchment A: Track & North D-Zone





Type III 24-hr 2-Year Storm Rainfall=3.20"

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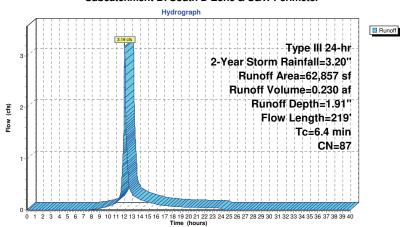
Summary for Subcatchment B: South D-Zone & S&W Perimeter

3.19 cfs @ 12.09 hrs, Volume= 0.230 af, Depth= 1.91" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Storm Rainfall=3.20"

	Α	rea (sf)	CN [Description		
*		21,236	98 F	Running Tra	ack	
*		403	98 F	Roof		
*		3,684	98 A	Asphalt Pav	/ement	
		37,534	79 5	0-75% Gra	ass cover, I	Fair, HSG C
_		62,857	87 V	Veighted A	verage	
		37,534	5	i9.71% Pei	vious Area	
		25,323	4	0.29% Imp	pervious Ar	ea
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.6	60	0.0290	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.2	32	0.0390	3.18		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	0.6	127	0.0420	3.30		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	6.4	219	Total			

Subcatchment B: South D-Zone & S&W Perimeter



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Type III 24-hr 2-Year Storm Rainfall=3.20" Printed 2/20/2019

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Summary for Subcatchment C: East Perimeter

0.86 cfs @ 12.12 hrs, Volume= 0.066 af, Depth= 1.61" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Storm Rainfall=3.20"

	(-1)	ON 5			
	rea (sf)	CN D	escription)		
*	4,828	98 A	sphalt Pav	/ement	
	16,717	79 5	0-75% Gra	ass cover, F	Fair, HSG C
	21,545	83 V	Veighted A	verage	
	16.717			vious Area	
	4.828	-		pervious Ar	
	4,020	_	2.71 /0 11116	oci vious 74i	ca
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Doddiption
	(/			(013)	Ob 4 Fl
6.2	60	0.0230	0.16		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
1.5	340	0.0050	3.72	4.57	
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Concrete pipe, bends & connections
0.2	56	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
0.5	143	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
0.0	1.10	0.0000	0.00	10.00	24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
					n= σ.στο συποτείε μίμε, bends & connections
8.4	599	Total			

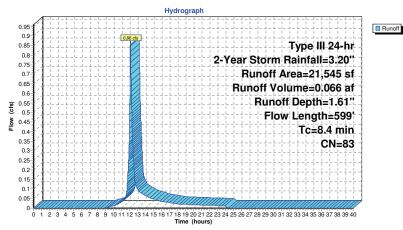
Type III 24-hr 2-Year Storm Rainfall=3.20"

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Subcatchment C: East Perimeter



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Summary for Subcatchment D: North Perimeter

0.113 af, Depth= 1.40" Runoff 1.41 cfs @ 12.14 hrs, Volume=

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Storm Rainfall=3.20"

	Α	rea (sf)	CN D	escription		
-	+	2,573	98 A	sphalt Pav	/ement	
		39,659	79 5	0-75% Gra	ass cover, F	Fair, HSG C
		42,232	80 V	/eighted A	verage	
		39,659	9	3.91% Per	vious Area	
		2,573	6	.09% Impe	ervious Are	a
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.5	60	0.0300	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.1	21	0.0500	3.60		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	0.5	98	0.0050	3.21	2.52	F
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
	4 7	000	0.0050	0.04	0.50	n= 0.013 Corrugated PE, smooth interior
	1.7	336	0.0050	3.21	2.52	
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
	1.5	336	0.0050	3.72	4.57	n= 0.013 Corrugated PE, smooth interior Pipe Channel, Existing 15" RCP
	1.5	330	0.0050	3.72	4.57	15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.013 Concrete pipe, bends & connections
-	9.3	051	Total			11- 0.010 Controlete pipe, bends & confidentions
	9.3	851	Total			

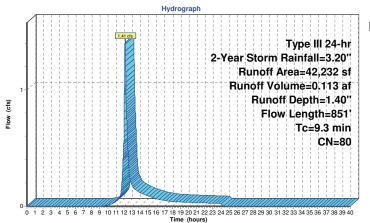
Type III 24-hr 2-Year Storm Rainfall=3.20"

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Subcatchment D: North Perimeter





Type III 24-hr 2-Year Storm Rainfall=3.20" 19014 PRE Printed 2/20/2019

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Summary for Pond 1P: YD

4.282 ac, 18.36% Impervious, Inflow Depth = 1.56" for 2-Year Storm event Inflow Area = 7.35 cfs @ 12.11 hrs, Volume= Inflow 0.557 af 7.35 cfs @ 12.11 hrs, Volume= Outflow 0.557 af, Atten= 0%, Lag= 0.0 min 7.35 cfs @ 12.11 hrs, Volume= Primary 0.557 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Peak Elev= 97.11' @ 12.11 hrs

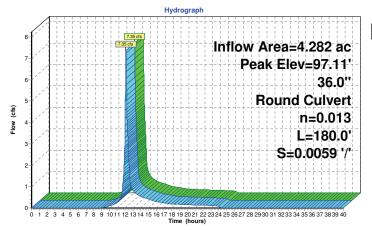
Flood Elev= 103.97'

Device Routing Invert Outlet Devices #1 Primary 96.02' 36.0" Round 36" RCP

L= 180.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 96.02' / 94.96' S= 0.0059 '/' Cc= 0.900 n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf

Primary OutFlow Max=7.34 cfs @ 12.11 hrs HW=97.11' (Free Discharge) 1=36" RCP (Barrel Controls 7.34 cfs @ 4.69 fps)

Pond 1P: YD





Type III 24-hr 2-Year Storm Rainfall=3.20"

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Summary for Link DP-1: South Wetland

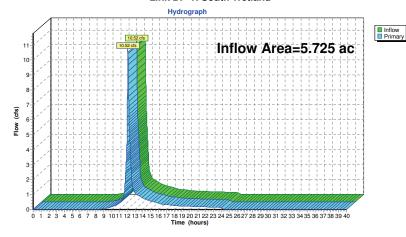
Inflow Area = 5.725 ac, 23.89% Impervious, Inflow Depth = 1.65" for 2-Year Storm event

Inflow 10.52 cfs @ 12.10 hrs, Volume= 0.788 af

10.52 cfs @ 12.10 hrs, Volume= 0.788 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Link DP-1: South Wetland



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Type III 24-hr 10-Year Storm Rainfall=4.70" Printed 2/20/2019

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Time span=0.00-40.00 hrs, dt=0.01 hrs, 4001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A: Track & North D-Zone Runoff Area=122,732 sf 21.87% Impervious Runoff Depth=2.90"

Flow Length=359' Tc=6.7 min CN=83 Runoff=9.34 cfs 0.682 af

Subcatchment B: South D-Zone & S&W Runoff Area=62,857 sf 40.29% Impervious Runoff Depth=3.29" Flow Length=219' Tc=6.4 min CN=87 Runoff=5.40 cfs 0.395 af

Subcatchment C: East Perimeter Runoff Area=21,545 sf 22.41% Impervious Runoff Depth=2.90"

Flow Length=599' Tc=8.4 min CN=83 Runoff=1.55 cfs 0.120 af

Subcatchment D: North Perimeter Runoff Area=42,232 sf 6.09% Impervious Runoff Depth=2.63" Flow Length=851' Tc=9.3 min CN=80 Runoff=2.67 cfs 0.213 af

Peak Elev=97.55' Inflow=13.40 cfs 1.014 af Pond 1P: YD

36.0" Round Culvert n=0.013 L=180.0' S=0.0059 '/' Outflow=13.40 cfs 1.014 af

Inflow=18.76 cfs 1.409 af Link DP-1: South Wetland Primary=18.76 cfs 1.409 af

> Total Runoff Area = 5.725 ac Runoff Volume = 1.409 af Average Runoff Depth = 2.95" 76.11% Pervious = 4.357 ac 23.89% Impervious = 1.367 ac

Type III 24-hr 10-Year Storm Rainfall=4.70"

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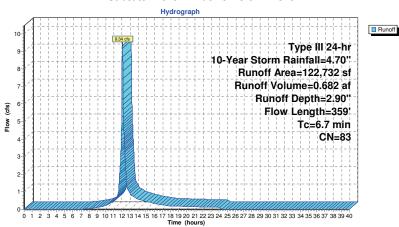
Summary for Subcatchment A: Track & North D-Zone

9.34 cfs @ 12.10 hrs, Volume= 0.682 af, Depth= 2.90" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Storm Rainfall=4.70"

_	Α	rea (sf)	CN E	escription		
*		26,841	98 F	Running Tra	ack	
		95,891	79 5	0-75% Gra	ass cover, I	Fair, HSG C
	1	22,732	83 V	Veighted A	verage	
		95,891	7	8.13% Per	vious Area	
		26,841	2	1.87% Imp	ervious Ar	ea
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.8	60	0.0420	0.21		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	1.4	156	0.0140	1.90		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	0.5	143	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
						24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
_						n= 0.013 Concrete pipe, bends & connections
	6.7	359	Total			

Subcatchment A: Track & North D-Zone



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Type III 24-hr 10-Year Storm Rainfall=4.70" Printed 2/20/2019

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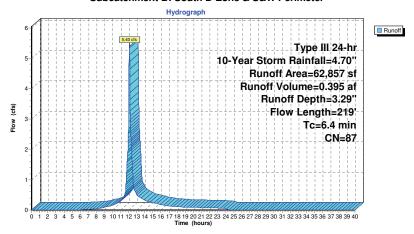
Summary for Subcatchment B: South D-Zone & S&W Perimeter

5.40 cfs @ 12.09 hrs, Volume= 0.395 af, Depth= 3.29" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Storm Rainfall=4.70"

	Д	rea (sf)	CN E	Description		
*		21,236	98 F	Running Tra	ack	
*		403	98 F	Roof		
*		3,684	98 A	sphalt Pav	/ement	
		37,534	79 5	0-75% Gra	ass cover, I	Fair, HSG C
		62,857	87 V	Veighted A	verage	
		37,534	5	9.71% Per	rvious Area	
		25,323	4	0.29% Imp	pervious Ar	ea
	_				_	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.6	60	0.0290	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.2	32	0.0390	3.18		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	0.6	127	0.0420	3.30		Shallow Concentrated Flow,
_						Unpaved Kv= 16.1 fps
	6.4	219	Total			

Subcatchment B: South D-Zone & S&W Perimeter



Type III 24-hr 10-Year Storm Rainfall=4.70"

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Summary for Subcatchment C: East Perimeter

1.55 cfs @ 12.12 hrs, Volume= 0.120 af, Depth= 2.90" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Storm Rainfall=4.70"

A	rea (sf)	CN D	escription		
*	4,828	98 A	sphalt Pav	rement	
	16,717	79 5	0-75% Gra	ass cover, I	Fair, HSG C
	21,545	83 V	Veighted A	verage	
	16,717	7	7.59% Per	vious Area	
	4,828	2	2.41% Imp	ervious Ar	ea
_					
Tc	Length	Slope	Velocity	Capacity	Description
(min)_	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.2	60	0.0230	0.16		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
1.5	340	0.0050	3.72	4.57	Pipe Channel, Existing 15" RCP
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Concrete pipe, bends & connections
0.2	56	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
0.5	143	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
8.4	599	Total			

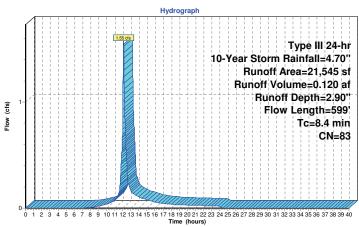
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Type III 24-hr 10-Year Storm Rainfall=4.70" Printed 2/20/2019

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Subcatchment C: East Perimeter





Type III 24-hr 10-Year Storm Rainfall=4.70"

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Summary for Subcatchment D: North Perimeter

Runoff 2.67 cfs @ 12.13 hrs, Volume= 0.213 af, Depth= 2.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Storm Rainfall=4.70"

	Α	rea (sf)	CN E	escription		
*		2.573	98 A	sphalt Pav	/ement	
		39,659				Fair, HSG C
_		42.232	80 V	Veighted A	verage	,
		39,659			vious Area	
		2,573	-		rvious Are	
		2,070		.00 /0 IIIIpc	7, 1,000 7,110	u
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
_	5.5	60	0.0300	0.18	(0.0)	Sheet Flow,
	0.0		0.0000	00		Grass: Short n= 0.150 P2= 3.20"
	0.1	21	0.0500	3.60		Shallow Concentrated Flow,
	٠		0.0000	0.00		Unpaved Kv= 16.1 fps
	0.5	98	0.0050	3.21	2.52	
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
						n= 0.013 Corrugated PE, smooth interior
	1.7	336	0.0050	3.21	2.52	
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
						n= 0.013 Corrugated PE, smooth interior
	1.5	336	0.0050	3.72	4.57	,
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.013 Concrete pipe, bends & connections
	9.3	851	Total			

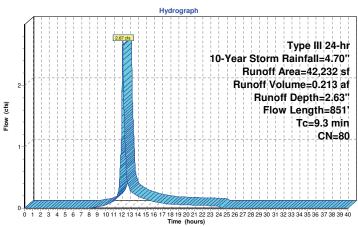
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Type III 24-hr 10-Year Storm Rainfall=4.70" Printed 2/20/2019

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Subcatchment D: North Perimeter





Type III 24-hr 10-Year Storm Rainfall=4.70"

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Inflow
Primary

Summary for Pond 1P: YD

4.282 ac, 18.36% Impervious, Inflow Depth = 2.84" for 10-Year Storm event Inflow Area =

Inflow 13.40 cfs @ 12.10 hrs, Volume= 1.014 af

Outflow 13.40 cfs @ 12.10 hrs, Volume= 1.014 af, Atten= 0%, Lag= 0.0 min

13.40 cfs @ 12.10 hrs, Volume= Primary 1.014 af

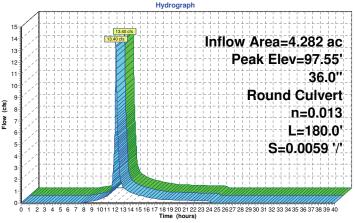
Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Peak Elev= 97.55' @ 12.10 hrs

Flood Elev= 103.97

Device	Routing	invert	Outlet Devices
#1	Primary	96.02'	36.0" Round 36" RCP
			L= 180.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 96.02' / 94.96' S= 0.0059 '/' Cc= 0.900
			n= 0.013 Concrete pine, hends & connections, Flow Area = 7.07 ef

Primary OutFlow Max=13.38 cfs @ 12.10 hrs HW=97.55' (Free Discharge) 1=36" RCP (Barrel Controls 13.38 cfs @ 5.40 fps)

Pond 1P: YD



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Type III 24-hr 10-Year Storm Rainfall=4.70" Printed 2/20/2019

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Summary for Link DP-1: South Wetland

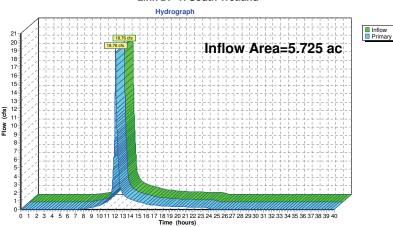
5.725 ac, 23.89% Impervious, Inflow Depth = 2.95" for 10-Year Storm event Inflow Area =

Inflow 18.76 cfs @ 12.10 hrs, Volume= 1.409 af

18.76 cfs @ 12.10 hrs, Volume= Primary = 1.409 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Link DP-1: South Wetland



Type III 24-hr 25-Year Storm Rainfall=5.50"

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Time span=0.00-40.00 hrs, dt=0.01 hrs, 4001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A: Track & North D-Zone	Runoff Area=122,732 sf	21.87% Impervious	Runoff Depth=3.63"
	Flow Longth 250! To 6.7	min CN 92 Dune	f 11 61 of 0 0 0 0 0 of

Subcatchment B: South D-Zone & S&W	Runoff Area=62,857 sf	40.29% Impervious	Runoff Depth=4.04"
	Flow Longth 210' To 6	4 min CN 97 Dun	off C EO ofo O 10C of

Subcatchment C: East Perimeter	Runoff Area=21,545 sf	22.41% Impervious	Runoff Depth=3.63"	
	Floor London FOOL To O	4 ON 00 D	-44 4 00 -4- 0 450 -4	

Subcatchment D: North Perimeter	Runoff Area=42,232 sf 6.09% Impervious Runoff Depth=3.33
	Flow I ength=851' Tc=9.3 min CN=80 Runoff=3.38 cfs 0.269.a

Pond 1P: YD	Peak Elev=97.76' Inflow=16.73 cfs 1.271	а
	36.0" Round Culvert, n=0.013, L=180.0', S=0.0059.1', Outflow=16.73.cfs, 1.271	a

Total Runoff Area = 5.725 ac Runoff Volume = 1.757 af Average Runoff Depth = 3.68" 76.11% Pervious = 4.357 ac 23.89% Impervious = 1.367 ac 19014 PRE

Type III 24-hr 25-Year Storm Rainfall=5.50" Printed 2/20/2019

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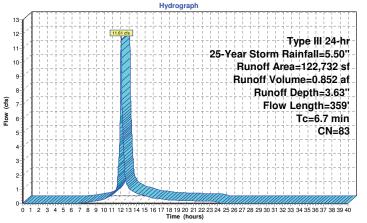
Summary for Subcatchment A: Track & North D-Zone

11.61 cfs @ 12.10 hrs, Volume= 0.852 af, Depth= 3.63" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Storm Rainfall=5.50"

	۸	rea (sf)	CN D	escription			
-							
*		26,841		Running Tra			
	95,891 79 50-75% Grass cover, Fair, HSG C						
	1	22.732	83 V	Veighted A	verage		
		95.891			vious Area		
		26.841	-		ervious Ar		
		20,041	_	1.07 /0 11116	ici vious i i	ca	
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
-	4.8	60	0.0420	0.21	(0.0)	Sheet Flow.	
	4.0	00	0.0720	0.21		Grass: Short n= 0.150 P2= 3.20"	
		450	0.04.40	4.00			
	1.4	156	0.0140	1.90		Shallow Concentrated Flow,	
						Unpaved Kv= 16.1 fps	
	0.5	143	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP	
						24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'	
						n= 0.013 Concrete pipe, bends & connections	
-	6.7	359	Total				

Subcatchment A: Track & North D-Zone





Type III 24-hr 25-Year Storm Rainfall=5.50"

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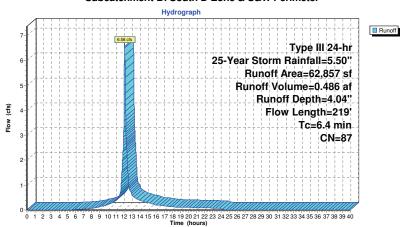
Summary for Subcatchment B: South D-Zone & S&W Perimeter

6.58 cfs @ 12.09 hrs, Volume= 0.486 af, Depth= 4.04" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Storm Rainfall=5.50"

	Α	rea (sf)	CN I	Description						
*		21,236	98 F	Running Track						
*		403	98 I	Roof						
*		3,684	98 /	Asphalt Pav	/ement					
		37,534	79 5	50-75% Grass cover, Fair, HSG C						
	62,857 87 Weighted Average									
		37,534	į	59.71% Per	vious Area					
		25,323	4	10.29% Imp	ervious Ar	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.6	60	0.0290	0.18		Sheet Flow,				
						Grass: Short n= 0.150 P2= 3.20"				
	0.2	32	0.0390	3.18		Shallow Concentrated Flow,				
						Unpaved Kv= 16.1 fps				
	0.6	127	0.0420	3.30		Shallow Concentrated Flow,				
_						Unpaved Kv= 16.1 fps				
	6.4	219	Total							

Subcatchment B: South D-Zone & S&W Perimeter



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Type III 24-hr 25-Year Storm Rainfall=5.50" Printed 2/20/2019

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Summary for Subcatchment C: East Perimeter

1.92 cfs @ 12.12 hrs, Volume= 0.150 af, Depth= 3.63" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Storm Rainfall=5.50"

Α	rea (sf)	CN E	escription		
*	4.828	98 A	sphalt Pav	/ement	
	16,717	Fair, HSG C			
	21,545		Veighted A		
	16,717			vious Area	
	4.828	2	2.41% Imr	pervious Ar	ea
	.,				
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
6.2	60	0.0230	0.16		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
1.5	340	0.0050	3.72	4.57	Pipe Channel, Existing 15" RCP
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Concrete pipe, bends & connections
0.2	56	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
0.5	143	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
8.4	599	Total			

Type III 24-hr 25-Year Storm Rainfall=5.50"

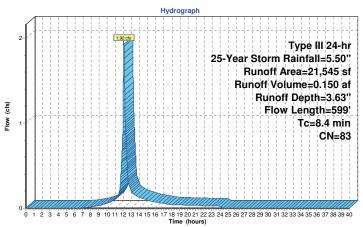
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Runoff

Subcatchment C: East Perimeter



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Type III 24-hr 25-Year Storm Rainfall=5.50" Printed 2/20/2019

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Summary for Subcatchment D: North Perimeter

Runoff 3.38 cfs @ 12.13 hrs, Volume= 0.269 af, Depth= 3.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Storm Rainfall=5.50"

	A	rea (sf)	CN D	escription		
-	,	2,573	98 A	sphalt Pav	/ement	
		39,659	79 5	0-75% Gra	ass cover, F	Fair, HSG C
-		42,232	80 V	Veighted A	verage	
		39,659	9	3.91% Per	vious Area	
		2,573	6	.09% Impe	ervious Area	a
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.5	60	0.0300	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.1	21	0.0500	3.60		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	0.5	98	0.0050	3.21	2.52	
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
	4 7	000	0.0050	0.04	0.50	n= 0.013 Corrugated PE, smooth interior
	1.7	336	0.0050	3.21	2.52	
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
	1.5	336	0.0050	3.72	4.57	n= 0.013 Corrugated PE, smooth interior Pipe Channel, Existing 15" RCP
	1.5	330	0.0050	3.72	4.57	15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.013 Concrete pipe, bends & connections
-	9.3	051	Total			11- 0.010 Condicte pipe, bends & confidentions
	9.3	851	Total			

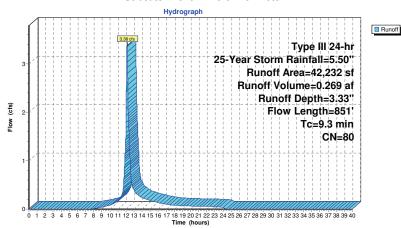
Type III 24-hr 25-Year Storm Rainfall=5.50"

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Subcatchment D: North Perimeter



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Type III 24-hr 25-Year Storm Rainfall=5.50"

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Summary for Pond 1P: YD

4.282 ac, 18.36% Impervious, Inflow Depth = 3.56" for 25-Year Storm event Inflow Area = 16.73 cfs @ 12.10 hrs, Volume= Inflow 1.271 af

16.73 cfs @ 12.10 hrs, Volume= Outflow 1.271 af, Atten= 0%, Lag= 0.0 min

16.73 cfs @ 12.10 hrs, Volume= Primary 1.271 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Peak Elev= 97.76' @ 12.10 hrs

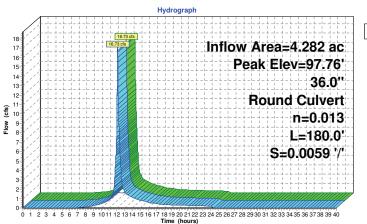
Flood Elev= 103.97'

Device Routing Invert Outlet Devices #1 Primary 96.02' 36.0" Round 36" RCP

L= 180.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 96.02' / 94.96' S= 0.0059 '/' Cc= 0.900 n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf

Primary OutFlow Max=16.70 cfs @ 12.10 hrs HW=97.76' (Free Discharge) 13.10 hrs HW=97.76' (Free Discharge) 13.70 cfs @ 5.68 fps)

Pond 1P: YD





Type III 24-hr 25-Year Storm Rainfall=5.50"

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Summary for Link DP-1: South Wetland

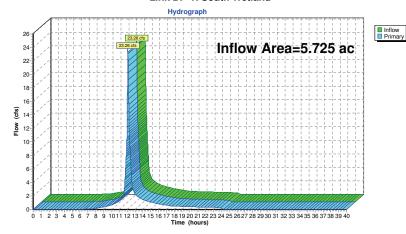
Inflow Area = 5.725 ac, 23.89% Impervious, Inflow Depth = 3.68" for 25-Year Storm event

Inflow 23.26 cfs @ 12.10 hrs, Volume= 1.757 af

23.26 cfs @ 12.10 hrs, Volume= 1.757 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Link DP-1: South Wetland



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Type III 24-hr 100-Year Storm Rainfall=6.90"

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Time span=0.00-40.00 hrs, dt=0.01 hrs, 4001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A: Track & North D-Zone Runoff Area=122,732 sf 21.87% Impervious Runoff Depth=4.93 Flow Length=359' Tc=6.7 min CN=83 Runoff=15.61 cfs 1.158 af

Subcatchment B: South D-Zone & S&W

Runoff Area=62,857 sf 40.29% Impervious Runoff Depth=5.38" Flow Length=219' Tc=6.4 min CN=87 Runoff=8.65 cfs 0.647 af

Subcatchment C: East Perimeter

Runoff Area=21,545 sf 22.41% Impervious Runoff Depth=4.93" Flow Length=599' Tc=8.4 min CN=83 Runoff=2.59 cfs 0.203 af

Subcatchment D: North Perimeter

Runoff Area=42,232 sf 6.09% Impervious Runoff Depth=4.60" Flow Length=851' Tc=9.3 min CN=80 Runoff=4.64 cfs 0.372 af

Pond 1P: YD

Peak Elev=98.10' Inflow=22.59 cfs 1.734 af

36.0" Round Culvert n=0.013 L=180.0' S=0.0059 '/' Outflow=22.59 cfs 1.734 af

Link DP-1: South Wetland

Inflow=31.17 cfs 2.381 af Primary=31.17 cfs 2.381 af

Total Runoff Area = 5.725 ac Runoff Volume = 2.381 af Average Runoff Depth = 4.99" 76.11% Pervious = 4.357 ac 23.89% Impervious = 1.367 ac

Type III 24-hr 100-Year Storm Rainfall=6.90"

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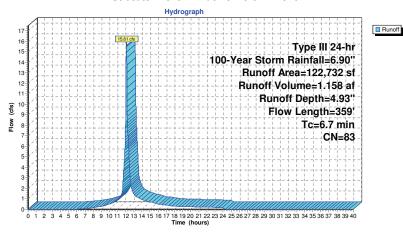
Summary for Subcatchment A: Track & North D-Zone

15.61 cfs @ 12.10 hrs, Volume= 1.158 af, Depth= 4.93" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Storm Rainfall=6.90"

	Α	rea (sf)	CN D	escription		
*		26,841	98 F	Running Tra	ack	
95.891 79 50-75% Grass cover, Fair, HSG C						
_	1	22,732	83 V	Veighted A	verage	
		95,891	7	8.13% Per	vious Area	
		26,841	2	1.87% Imp	ervious Ar	ea
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.8	60	0.0420	0.21		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	1.4	156	0.0140	1.90		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	0.5	143	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
						24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
_						n= 0.013 Concrete pipe, bends & connections
	6.7	359	Total			

Subcatchment A: Track & North D-Zone



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Type III 24-hr 100-Year Storm Rainfall=6.90" Printed 2/20/2019

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Summary for Subcatchment B: South D-Zone & S&W Perimeter

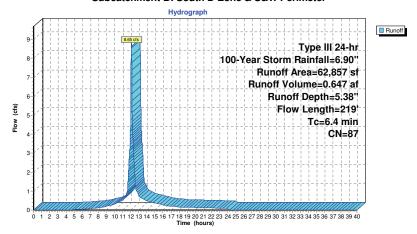
8.65 cfs @ 12.09 hrs, Volume= Runoff

0.647 af, Depth= 5.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Storm Rainfall=6.90"

	P	Area (sf)	CN [Description					
*		21.236	98 F	Running Tr	ack				
*		403		Roof					
*		3,684	98 A	Asphalt Pav	sphalt Pavement				
		37,534	79 5	50-75% Gra	ass cover, I	Fair, HSG C			
		62,857	87 V	Weighted A	verage				
		37,534	5	59.71% Pei	rvious Area				
		25,323	4	10.29% Imp	pervious Ar	ea			
	Tc	Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.6	60	0.0290	0.18		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.20"			
	0.2	32	0.0390	3.18		Shallow Concentrated Flow,			
						Unpaved Kv= 16.1 fps			
	0.6	127	0.0420	3.30		Shallow Concentrated Flow,			
_						Unpaved Kv= 16.1 fps			
	6.4	219	Total						

Subcatchment B: South D-Zone & S&W Perimeter



Type III 24-hr 100-Year Storm Rainfall=6.90"

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Summary for Subcatchment C: East Perimeter

2.59 cfs @ 12.12 hrs, Volume= 0.203 af, Depth= 4.93" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Storm Rainfall=6.90"

A	rea (sf)	CN D	escription		
*	4,828	98 A	sphalt Pav	/ement	
	16,717	79 5	0-75% Gra	ass cover, I	Fair, HSG C
	21,545	83 V	Veighted A	verage	
	16,717	7	7.59% Per	vious Area	
	4,828	2	2.41% Imp	ervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.2	60	0.0230	0.16		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
1.5	340	0.0050	3.72	4.57	Pipe Channel, Existing 15" RCP
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Concrete pipe, bends & connections
0.2	56	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
0.5	143	0.0050	5.09	16.00	Pipe Channel, Existing 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
8.4	599	Total			

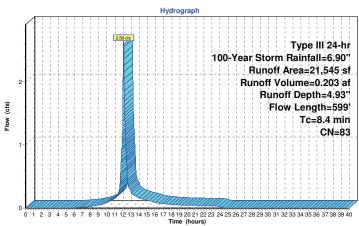
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Type III 24-hr 100-Year Storm Rainfall=6.90" Printed 2/20/2019

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Subcatchment C: East Perimeter





Type III 24-hr 100-Year Storm Rainfall=6.90"

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Summary for Subcatchment D: North Perimeter

4.64 cfs @ 12.13 hrs, Volume= 0.372 af, Depth= 4.60" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Storm Rainfall=6.90"

^	rea (sf)	CN E	escription		
A					
*	2,573		sphalt Pa۱،		
	39,659	79 5	0-75% Gra	ass cover, I	Fair, HSG C
	42,232	80 V	Veighted A	verage	
	39,659	9	3.91% Per	vious Area	
	2,573	6	.09% Impe	rvious Are	a
	,				
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	The second secon
5.5	60	0.0300	0.18	` '	Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
0.1	21	0.0500	3.60		Shallow Concentrated Flow,
					Unpayed Ky= 16.1 fps
0.5	98	0.0050	3.21	2.52	- Production of the
0.0	00	0.0000	0.2.	2.02	12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013 Corrugated PE, smooth interior
1.7	336	0.0050	3.21	2.52	
1.7	000	0.0000	0.21	2.52	12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013 Corrugated PE, smooth interior
1.5	336	0.0050	3.72	4 57	,
1.5	336	0.0050	3.72	4.57	
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Concrete pipe, bends & connections
9.3	851	Total			

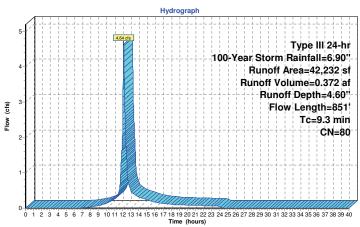
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Type III 24-hr 100-Year Storm Rainfall=6.90" Printed 2/20/2019

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Subcatchment D: North Perimeter





Type III 24-hr 100-Year Storm Rainfall=6.90"

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Summary for Pond 1P: YD

4.282 ac, 18.36% Impervious, Inflow Depth = 4.86" for 100-Year Storm event Inflow Area =

Inflow 22.59 cfs @ 12.10 hrs, Volume= 1.734 af

Outflow 22.59 cfs @ 12.10 hrs, Volume= 1.734 af, Atten= 0%, Lag= 0.0 min

Primary 22.59 cfs @ 12.10 hrs, Volume= 1.734 af

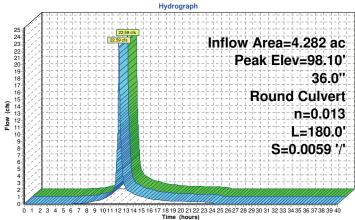
Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Peak Elev= 98.10' @ 12.10 hrs

Flood Elev= 103.97

Device	Routing	Invert	Outlet Devices
#1	Primary	96.02'	36.0" Round 36" RCP
			L= 180.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 96.02' / 94.96' S= 0.0059 '/' Cc= 0.900
			n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf

Pond 1P: YD

Primary OutFlow Max=22.56 cfs @ 12.10 hrs HW=98.10' (Free Discharge) 1=36" RCP (Barrel Controls 22.56 cfs @ 6.07 fps)



Inflow
Primary

19014 PRE

Type III 24-hr 100-Year Storm Rainfall=6.90"

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Summary for Link DP-1: South Wetland

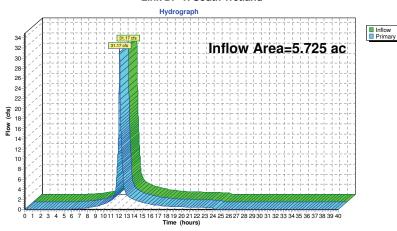
5.725 ac, 23.89% Impervious, Inflow Depth = 4.99" for 100-Year Storm event Inflow Area =

Inflow 31.17 cfs @ 12.10 hrs, Volume= 2.381 af

31.17 cfs @ 12.10 hrs, Volume= Primary = 2.381 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Link DP-1: South Wetland



University of Saint Joseph – Athletic Field Renovations

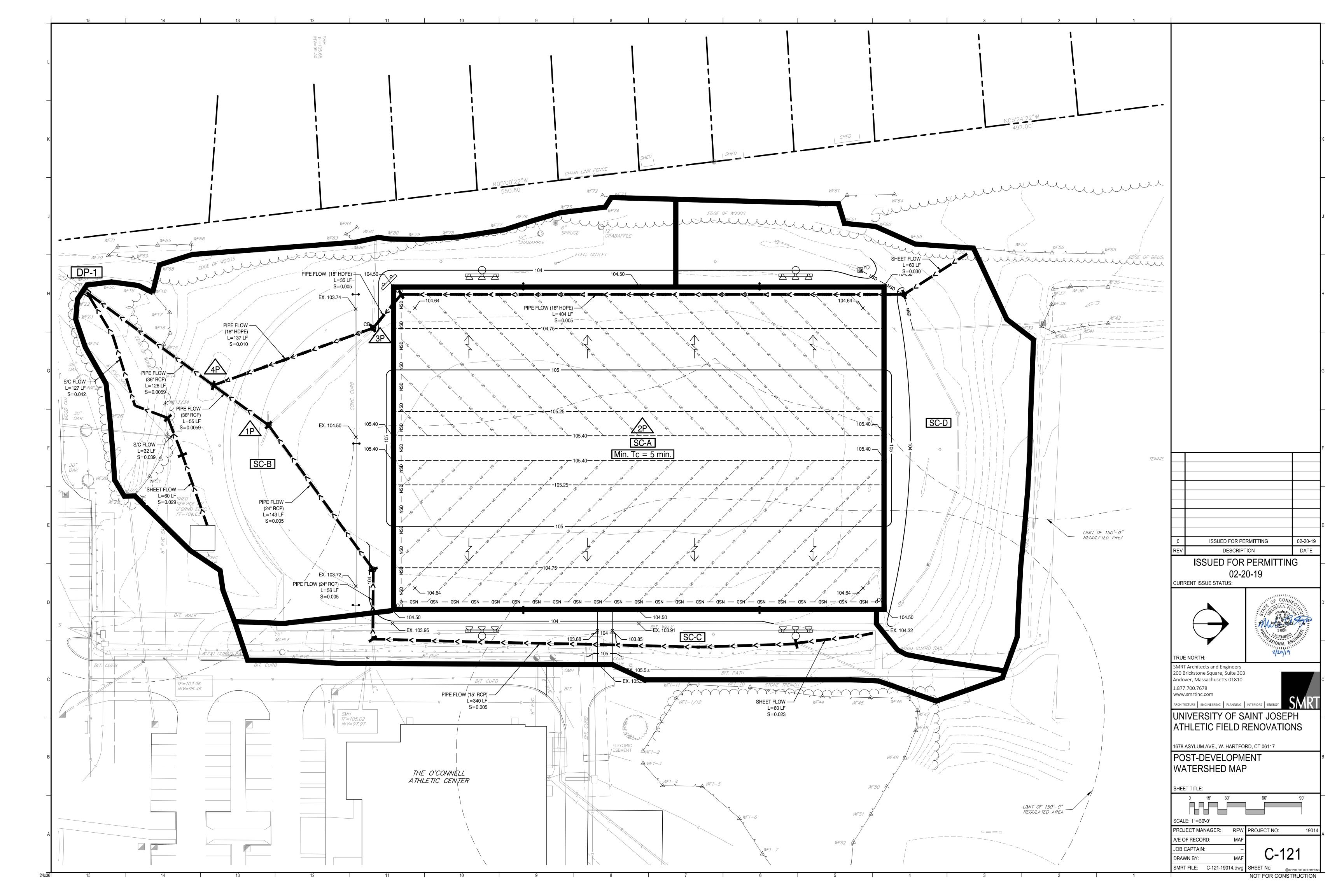
Stormwater Management Report

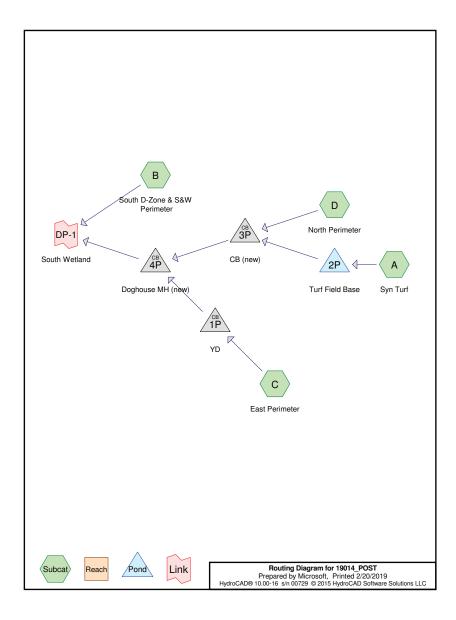
Post-Development Conditions Analysis

Appendix D

CONTENTS:

- 1. Post-development Watershed Map
- 2. Post-development HydroCAD Runoff and Routing Calculations





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Area Listing (all nodes)

(Area (acres)	CN	Description (subcatchment-numbers)
	2.717	79	50-75% Grass cover, Fair, HSG C (B, C, D)
	0.176	98	Asphalt Pavement (B, C)
	0.009	98	Roof (B)
	0.480	98	Running Track (B)
	2.343	98	Synthetic Turf (A)
	5.725	89	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
2.717	HSG C	B, C, D
0.000	HSG D	
3.008	Other	A, B, C
5.725		TOTAL AREA

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	2.717	0.000	0.000	2.717	50-75% Grass cover, Fair	B, C, D
0.000	0.000	0.000	0.000	0.176	0.176	Asphalt Pavement	B, C
0.000	0.000	0.000	0.000	0.009	0.009	Roof	В
0.000	0.000	0.000	0.000	0.480	0.480	Running Track	В
0.000	0.000	0.000	0.000	2.343	2.343	Synthetic Turf	Α
0.000	0.000	2.717	0.000	3.008	5.725	TOTAL AREA	

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Pipe Listing (all nodes)

Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Diam/Width	Height	Inside-Fill
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)
1	С	0.00	0.00	396.0	0.0050	0.013	15.0	0.0	0.0
2	С	0.00	0.00	56.0	0.0050	0.013	24.0	0.0	0.0
3	С	0.00	0.00	143.0	0.0050	0.013	24.0	0.0	0.0
4	D	0.00	0.00	404.0	0.0050	0.013	18.0	0.0	0.0
5	D	0.00	0.00	35.0	0.0050	0.013	18.0	0.0	0.0
6	1P	96.02	95.70	55.0	0.0058	0.013	36.0	0.0	0.0
7	2P	97.28	97.10	35.0	0.0051	0.013	18.0	0.0	0.0
8	3P	97.01	95.64	137.0	0.0100	0.013	18.0	0.0	0.0
9	4P	95.70	94.96	126.0	0.0059	0.013	36.0	0.0	0.0

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Type III 24-hr 1-Year Storm Rainfall=2.60"

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Time span=0.00-40.00 hrs, dt=0.01 hrs, 4001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A: Syn Turf Runoff Area=102,046 sf 100.00% Impervious Runoff Depth=2.37"

Tc=5.0 min CN=98 Runoff=6.09 cfs 0.463 af

Subcatchment B: South D-Zone & S&W Runoff Area=73,634 sf 32.24% Impervious Runoff Depth=1.26" Flow Length=219' Tc=6.4 min CN=85 Runoff=2.45 cfs 0.177 af

Subcatchment C: East Perimeter

Runoff Area=26,551 sf 19.75% Impervious Runoff Depth=1.13"
Flow Length=655' Tc=8.7 min CN=83 Runoff=0.73 cfs 0.057 af

Subcatchment D: North Perimeter Runoff Area=47,135 sf 0.00% Impervious Runoff Depth=0.91"

Flow Length=499' Tc=7.2 min CN=79 Runoff=1.06 cfs 0.082 af

Pond 1P: YD Peak Elev=96.37' Inflow=0.73 cfs 0.057 af

36.0" Round Culvert n=0.013 L=55.0' S=0.0058 '/' Outflow=0.73 cfs 0.057 af

Pond 2P: Turf Field Base Peak Elev=103.56' Storage=17,040 cf Inflow=6.09 cfs 0.463 af

Outflow=0.14 cfs 0.105 af

Pond 3P: CB (new) Peak Elev=97.53' Inflow=1.06 cfs 0.187 af

18.0" Round Culvert n=0.013 L=137.0' S=0.0100 '/' Outflow=1.06 cfs 0.187 af

Pond 4P: Doghouse MH (new) Peak Elev=96.23' Inflow=1.78 cfs 0.244 af

36.0" Round Culvert n=0.013 L=126.0' S=0.0059 '/' Outflow=1.78 cfs 0.244 af

Link DP-1: South Wetland Inflow=4.20 cfs 0.422 af

Primary=4.20 cfs 0.422 af

Total Runoff Area = 5.725 ac Runoff Volume = 0.779 af Average Runoff Depth = 1.63" 47.45% Pervious = 2.717 ac 52.55% Impervious = 3.008 ac

Type III 24-hr 1-Year Storm Rainfall=2.60"

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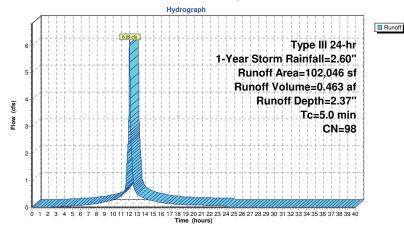
Summary for Subcatchment A: Syn Turf

6.09 cfs @ 12.07 hrs, Volume= 0.463 af, Depth= 2.37" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Storm Rainfall=2.60"

	Α	rea (sf)	CN [Description						
3	1	02,046	98 3	Synthetic To	ynthetic Turf					
	102,046 100.00% Impervious Ar			00.00% Im	pervious A	urea				
	Tc	Length	Slope		Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.0					Direct Entry, Min. Tc				

Subcatchment A: Syn Turf



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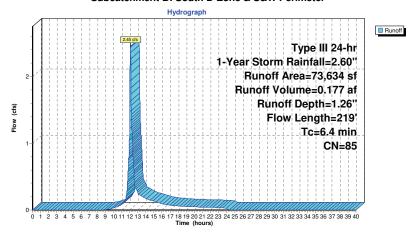
Summary for Subcatchment B: South D-Zone & S&W Perimeter

2.45 cfs @ 12.10 hrs, Volume= 0.177 af, Depth= 1.26" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Storm Rainfall=2.60"

Α	rea (sf)	CN [Description		
*	20.920	98 F	Running Tra	ack	
*	403		Roof		
*	2.419		Asphalt Pav	ement	
	49,892				Fair, HSG C
	73.634		Veighted A		4.1,1.00
	49.892			vious Area	
	- ,				
	23,742		32.24% IIII	ervious Ar	ea
Tc	Longth	Clana	Volonity	Consoitu	Description
	Length	Slope		Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.6	60	0.0290	0.18		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
0.2	32	0.0390	3.18		Shallow Concentrated Flow,
					Unpaved Kv= 16.1 fps
0.6	127	0.0420	3.30		Shallow Concentrated Flow,
0.0		0.0.20	0.00		Unpaved Kv= 16.1 fps
C 4	010	Tatal			011paroa 114- 10.1 1po
6.4	219	Total			

Subcatchment B: South D-Zone & S&W Perimeter



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Type III 24-hr 1-Year Storm Rainfall=2.60"

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Summary for Subcatchment C: East Perimeter

Runoff = 0.73 cfs @ 12.13 hrs, Volume= 0.057 af, Depth= 1.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Storm Rainfall=2.60"

Α	rea (sf)	CN D	escription		
*	5.244	98 A	sphalt Pav	/ement	
	21,307				Fair, HSG C
	26,551	83 V	Veighted A	verage	
	21,307			vious Area	
	5,244	1	9.75% Imp	ervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.2	60	0.0230	0.16		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
1.8	396	0.0050	3.72	4.57	
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Concrete pipe, bends & connections
0.2	56	0.0050	5.09	16.00	Pipe Channel, 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
0.5	143	0.0050	5.09	16.00	
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
8.7	655	Total			

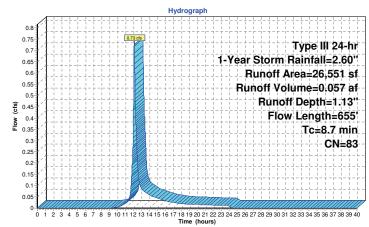
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Subcatchment C: East Perimeter





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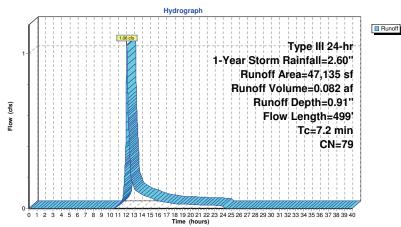
Summary for Subcatchment D: North Perimeter

0.082 af, Depth= 0.91" Runoff 1.06 cfs @ 12.11 hrs, Volume=

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Storm Rainfall=2.60"

	Α	rea (sf)	CN [Description					
		47,135	79 50-75% Grass cover, Fair, HSG C						
	47,135 100.00% Pervious Area					ea .			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
-	5.5	60	0.0300	0.18		Sheet Flow,			
	1.6	404	0.0050	4.20	7.43	18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'			
	0.1	35	0.0050	4.20	7.43	n= 0.013 Corrugated PE, smooth interior Pipe Channel, 18" HDPE 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013 Corrugated PE, smooth interior			
-	7.2	400	Total						

Subcatchment D: North Perimeter



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Summary for Pond 1P: YD

0.610 ac, 19.75% Impervious, Inflow Depth = 1.13" for 1-Year Storm event Inflow Area = Inflow 0.73 cfs @ 12.13 hrs, Volume= 0.057 af

Outflow 0.73 cfs @ 12.13 hrs, Volume= 0.057 af, Atten= 0%, Lag= 0.0 min

0.73 cfs @ 12.13 hrs, Volume= Primary 0.057 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Peak Elev= 96.37' @ 12.13 hrs

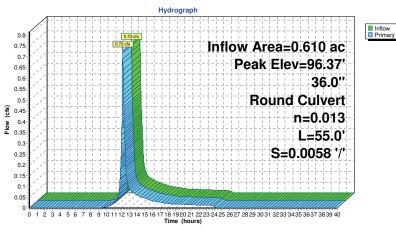
Flood Elev= 103.97

Device Routing Invert Outlet Devices #1 Primary 96.02' 36.0" Round 36" RCP L= 55.0' RCP, sq.cut end projecting, Ke= 0.500

Inlet / Outlet Invert= 96.02' / 95.70' S= 0.0058 '/' Cc= 0.900 n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf

Primary OutFlow Max=0.72 cfs @ 12.13 hrs HW=96.37' (Free Discharge) 1=36" RCP (Barrel Controls 0.72 cfs @ 2.42 fps)

Pond 1P: YD



#2 Device 1

Type III 24-hr 1-Year Storm Rainfall=2.60"

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Summary for Pond 2P: Turf Field Base

Inflow Area = 2.343 ac,100.00% Impervious, Inflow Depth = 2.37" for 1-Year Storm event Inflow 6.09 cfs @ 12.07 hrs, Volume= 0.463 af Outflow 0.14 cfs @ 16.82 hrs, Volume= 0.105 af, Atten= 98%, Lag= 284.7 min Primary 0.14 cfs @ 16.82 hrs, Volume= 0.105 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Peak Elev= 103.56' @ 16.82 hrs Surf.Area= 102,046 sf Storage= 17,040 cf Plug-Flow detention time= 703.8 min calculated for 0.105 af (23% of inflow) Center-of-Mass det. time= 488.8 min (1,248.8 - 760.1)

Volume Invert Avail.Storage Storage Description 45,921 cf Custom Stage Data (Prismatic) Listed below (Recalc) #1 103.00' Inc.Store Elevation Surf.Area Voids Cum.Store (feet) (sq-ft) (%) (cubic-feet) (cubic-feet) 102,046 0.0 103.00 15 207

103.50		00	102,046 30	.0 15,307	15,307				
	104.50		102.046 30	.0 30.614	45.921				
		-	,		,				
	Device	Routing	Invert	Outlet Devices					
	#1	Primary	97.28'	18.0" Round 18" HDF	PE				
		•		L= 35.0' CPP, projecting, no headwall, Ke= 0.900					
				Inlet / Outlet Invert= 97.28' / 97.10' S= 0.0051 '/' Cc= 0.900					
				n= 0.013 Corrugated I	PE, smooth interior, Flow Area= 1.77 sf				

103.50' 4.0" Vert. Orifice/Grate X 17.00 C= 0.600

Primary OutFlow Max=0.14 cfs @ 16.82 hrs HW=103.56' (Free Discharge) 1=18" HDPE (Passes 0.14 cfs of 15.79 cfs potential flow) 2=Orifice/Grate (Orifice Controls 0.14 cfs @ 0.81 fps)

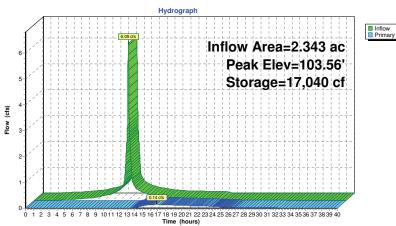
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Pond 2P: Turf Field Base



Type III 24-hr 1-Year Storm Rainfall=2.60"

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Summary for Pond 3P: CB (new)

[79] Warning: Submerged Pond 2P Primary device # 1 INLET by 0.25'

Inflow Area = 3.425 ac, 68.40% Impervious, Inflow Depth > 0.65" for 1-Year Storm event

Inflow = 1.06 cfs @ 12.11 hrs, Volume= 0.187 af

Outflow = 1.06 cfs @ 12.11 hrs, Volume= 0.187 af, Atten= 0%, Lag= 0.0 min

Primary = 1.06 cfs @ 12.11 hrs, Volume= 0.187 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Peak Elev= 97.53' @ 12.11 hrs

Flood Elev= 103.70'

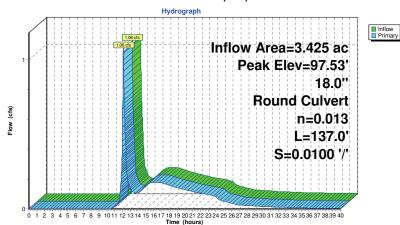
Device	Routing	Invert	Outlet Devices
#1	Primary	97.01'	18.0" Round 18" HDPE L= 137.0' CPP, projecting, no headwall, Ke= 0.900 high! / Outlet Invert = 97.01' / 95.64' S= 0.0100'/' Cc= 0.900

n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=1.05 cfs @ 12.11 hrs HW=97.53' (Free Discharge)

1=18" HDPE (Inlet Controls 1.05 cfs @ 1.94 fps)

Pond 3P: CB (new)



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Summary for Pond 4P: Doghouse MH (new)

[79] Warning: Submerged Pond 1P Primary device # 1 INLET by 0.21' [79] Warning: Submerged Pond 3P Primary device # 1 OUTLET by 0.59'

Inflow Area = 4.034 ac, 61.05% Impervious, Inflow Depth > 0.73" for 1-Year Storm event

Inflow = 1.78 cfs @ 12.12 hrs, Volume= 0.244 af

Outflow = 1.78 cfs @ 12.12 hrs, Volume= 0.244 af, Atten= 0%, Lag= 0.0 min

Primary = 1.78 cfs @ 12.12 hrs, Volume= 0.244 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

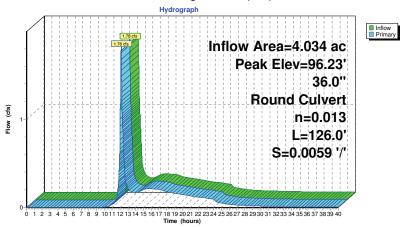
Peak Elev= 96.23' @ 12.12 hrs

Flood Elev= 103.97

Device	Routing	Invert	Outlet Devices					
#1	Primary	95.70'	36.0" Round 36" RCP					
			L= 126.0' RCP, sq.cut end projecting, Ke= 0.500					
			nlet / Outlet Invert= 95.70' / 94.96' S= 0.0059 '/' Cc= 0.900					
			n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf					

Primary OutFlow Max=1.77 cfs @ 12.12 hrs HW=96.23' (Free Discharge) 1=36" RCP (Barrel Controls 1.77 cfs @ 3.19 fps)

Pond 4P: Doghouse MH (new)



Type III 24-hr 1-Year Storm Rainfall=2.60"

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Summary for Link DP-1: South Wetland

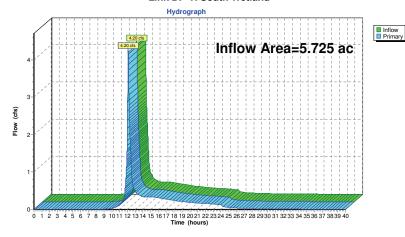
5.725 ac, 52.55% Impervious, Inflow Depth > 0.88" for 1-Year Storm event Inflow Area =

Inflow 4.20 cfs @ 12.10 hrs, Volume= 0.422 af

4.20 cfs @ 12.10 hrs, Volume= 0.422 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Link DP-1: South Wetland



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Pond 2P: Turf Field Base

Type III 24-hr 2-Year Storm Rainfall=3.20"

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Time span=0.00-40.00 hrs, dt=0.01 hrs, 4001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Runoff Area=102.046 sf 100.00% Impervious Runoff Depth=2.97" Subcatchment A: Syn Turf

Tc=5.0 min CN=98 Runoff=7.54 cfs 0.579 af

Runoff Area=73,634 sf 32.24% Impervious Runoff Depth=1.76" Subcatchment B: South D-Zone & S&W Flow Length=219' Tc=6.4 min CN=85 Runoff=3.44 cfs 0.248 af

Subcatchment C: East Perimeter Runoff Area=26,551 sf 19.75% Impervious Runoff Depth=1.61"

Flow Length=655' Tc=8.7 min CN=83 Runoff=1.05 cfs 0.082 af

Subcatchment D: North Perimeter Runoff Area=47,135 sf 0.00% Impervious Runoff Depth=1.34" Flow Length=499' Tc=7.2 min CN=79 Runoff=1.60 cfs 0.121 af

Pond 1P: YD Peak Elev=96.44' Inflow=1.05 cfs 0.082 af 36.0" Round Culvert n=0.013 L=55.0' S=0.0058 '/' Outflow=1.05 cfs 0.082 af

Peak Elev=103.60' Storage=18,305 cf Inflow=7.54 cfs 0.579 af Outflow=0.39 cfs 0.222 af

Peak Elev=97.66' Inflow=1.60 cfs 0.342 af Pond 3P: CB (new)

18.0" Round Culvert n=0.013 L=137.0' S=0.0100 '/' Outflow=1.60 cfs 0.342 af

Peak Elev=96.35' Inflow=2.64 cfs 0.424 af Pond 4P: Doghouse MH (new)

36.0" Round Culvert n=0.013 L=126.0' S=0.0059 '/' Outflow=2.64 cfs 0.424 af

Inflow=6.03 cfs 0.672 af Link DP-1: South Wetland Primary=6.03 cfs 0.672 af

> Total Runoff Area = 5.725 ac Runoff Volume = 1.029 af Average Runoff Depth = 2.16" 47.45% Pervious = 2.717 ac 52.55% Impervious = 3.008 ac

Type III 24-hr 2-Year Storm Rainfall=3.20"

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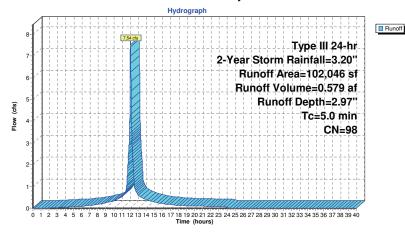
Summary for Subcatchment A: Syn Turf

7.54 cfs @ 12.07 hrs, Volume= 0.579 af, Depth= 2.97" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Storm Rainfall=3.20"

	Α	rea (sf)	CN I	Description					
4	1	02,046	98 3	Synthetic Turf					
	1	02,046		100.00% Impervious Area					
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.0					Direct Entry, Min. Tc			

Subcatchment A: Syn Turf



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Type III 24-hr 2-Year Storm Rainfall=3.20"

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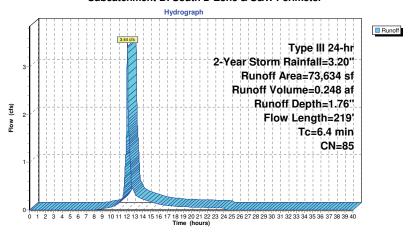
Summary for Subcatchment B: South D-Zone & S&W Perimeter

3.44 cfs @ 12.09 hrs, Volume= 0.248 af, Depth= 1.76" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Storm Rainfall=3.20"

	Δ	rea (sf)	CN E	Description				
*		20.920		Running Tra				
*		403		Roof				
*		2,419	98 <i>F</i>	Asphalt Pav	/ement			
		49,892	79 5	0-75% Gra	ass cover, F	Fair, HSG C		
		73,634		Veighted A				
		49,892	-		vious Area			
		23,742	3	32.24% Impervious Area				
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	5.6	60	0.0290	0.18		Sheet Flow,		
						Grass: Short n= 0.150 P2= 3.20"		
	0.2	32	0.0390	3.18	3.18 Shallow Concentrated Flow,			
						Unpaved Kv= 16.1 fps		
	0.6	127	0.0420	3.30		Shallow Concentrated Flow,		
_						Unpaved Kv= 16.1 fps		
	6.4	219	Total					

Subcatchment B: South D-Zone & S&W Perimeter



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Type III 24-hr 2-Year Storm Rainfall=3.20"

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Summary for Subcatchment C: East Perimeter

1.05 cfs @ 12.13 hrs, Volume= 0.082 af, Depth= 1.61" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Storm Rainfall=3.20"

A	rea (sf)	CN D	escription						
*	5,244	98 A	98 Asphalt Pavement						
	21,307	79 5							
	26,551 83 Weighted Average								
	21,307	8	0.25% Per	vious Area					
	5,244	1	19.75% Impervious Area						
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.2	60	0.0230	0.16		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.20"				
1.8	396	0.0050	3.72	4.57	Pipe Channel, 15" RCP				
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'				
					n= 0.013 Concrete pipe, bends & connections				
0.2	56	0.0050	5.09	16.00	Pipe Channel, 24" RCP				
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'				
					n= 0.013 Concrete pipe, bends & connections				
0.5	143	0.0050	5.09	16.00	Pipe Channel, 24" RCP				
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'				
					n= 0.013 Concrete pipe, bends & connections				
8.7	655	Total							

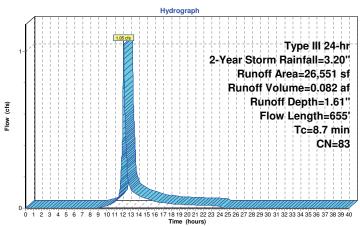
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Subcatchment C: East Perimeter





Type III 24-hr 2-Year Storm Rainfall=3.20"

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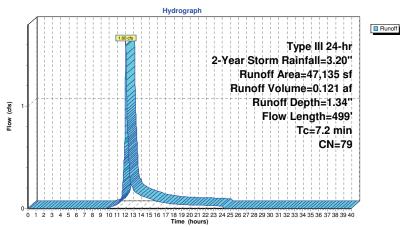
Summary for Subcatchment D: North Perimeter

Runoff = 1.60 cfs @ 12.11 hrs, Volume= 0.121 af, Depth= 1.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Storm Rainfall=3.20"

		(-6)	ON	D				
_	A	rea (sf)	CN	Description				
	47,135 79 50-75% Grass cover, Fair, HSG C							
	47,135			100.00% P	ervious Are	a		
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
	5.5	60	0.0300	0.18		Sheet Flow,		
	1.6	404	0.0050	4.20	7.43	Grass: Short n= 0.150 P2= 3.20" Pipe Channel, 18" HDPE Collector Pipe 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'		
	0.1	35	0.0050	4.20	7.43	n= 0.013 Corrugated PE, smooth interior Pipe Channel, 18" HDPE 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013 Corrugated PE, smooth interior		
	7.2	400	Total					

Subcatchment D: North Perimeter



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Summary for Pond 1P: YD

Outflow = 1.05 cfs @ 12.13 hrs, Volume= 0.082 af, Atten= 0%, Lag= 0.0 min

Primary = 1.05 cfs @ 12.13 hrs, Volume= 0.082 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Peak Elev= 96.44' @ 12.13 hrs

Flood Elev= 103.97

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 96.02'
 36.0" Round 36" RCP

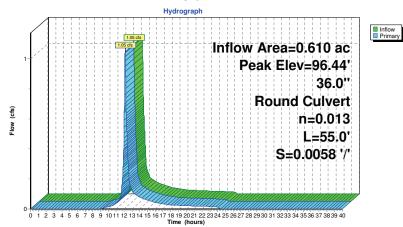
 L = 55.0'
 RCP, sq.cut end projecting, Ke= 0.500

 Inlet / Outlet Invert= 96.02' / 95.70'
 S= 0.0058 '/' Cc= 0.900

 n= 0.013
 Concrete pipe, bends & connections, Flow Area= 7.07 sf

Primary OutFlow Max=1.04 cfs @ 12.13 hrs HW=96.44' (Free Discharge) 1=36" RCP (Barrel Controls 1.04 cfs @ 2.66 fps)

Pond 1P: YD



Volume

Type III 24-hr 2-Year Storm Rainfall=3.20"

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Summary for Pond 2P: Turf Field Base

2.343 ac,100.00% Impervious, Inflow Depth = 2.97" for 2-Year Storm event Inflow Area = Inflow 7.54 cfs @ 12.07 hrs, Volume= 0.579 af

Outflow 0.39 cfs @ 13.97 hrs, Volume= 0.222 af, Atten= 95%, Lag= 113.8 min

0.39 cfs @ 13.97 hrs, Volume= 0.222 af Primary

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Peak Elev= 103.60' @ 13.97 hrs Surf.Area= 102,046 sf Storage= 18,305 cf

Plug-Flow detention time= 482.4 min calculated for 0.222 af (38% of inflow)

Avail.Storage Storage Description

Center-of-Mass det. time= 329.2 min (1,084.7 - 755.5)

Invert

#1	103.0	00'	45,921 cf	Custom Stage I	Data (Prismatic) L	isted below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
103.0		102,046	0.0	0	0	
103.50		102,046	30.0	15,307	15,307	
104.50		102,046	30.0	30,614	45,921	
Device	Routing	Ir	nvert Out	let Devices		
#1	Primary	9	7.28' 18. 0)" Round 18" HD	PE	
	,		L= :	35.0' CPP, project	cting, no headwall.	Ke= 0.900
						0.0051 '/' Cc= 0.900
			n=	0.013 Corrugated	PF smooth interi	or, Flow Area= 1.77 sf
#2	Device 1	100		' Vert. Orifice/Gra		

Primary OutFlow Max=0.39 cfs @ 13.97 hrs HW=103.60' (Free Discharge)
1=18" HDPE (Passes 0.39 cfs of 15.85 cfs potential flow)
2=Orifice/Grate (Orifice Controls 0.39 cfs @ 1.07 fps)

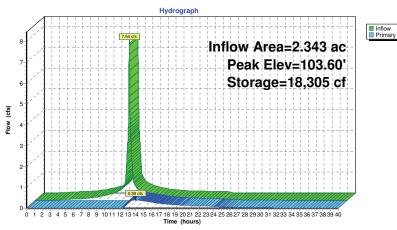
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Pond 2P: Turf Field Base



Type III 24-hr 2-Year Storm Rainfall=3.20"

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Summary for Pond 3P: CB (new)

[79] Warning: Submerged Pond 2P Primary device # 1 INLET by 0.38'

Inflow Area = 3.425 ac, 68.40% Impervious, Inflow Depth > 1.20" for 2-Year Storm event

Inflow = 1.60 cfs @ 12.11 hrs, Volume= 0.342 af

Outflow = 1.60 cfs @ 12.11 hrs, Volume= 0.342 af, Atten= 0%, Laq= 0.0 min

Primary = 1.60 cfs @ 12.11 hrs, Volume= 0.342 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Peak Elev= 97.66' @ 12.11 hrs

Flood Elev= 103.70'

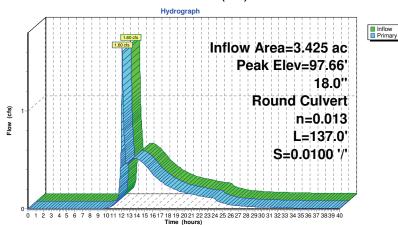
Device	Routing	Invert	Outlet Devices
#1	Primary	97.01'	18.0" Round 18" HDPE
			L= 137.0' CPP, projecting, no headwall, Ke= 0.900

n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=1.60 cfs @ 12.11 hrs HW=97.66' (Free Discharge)

1=18" HDPE (Inlet Controls 1.60 cfs @ 2.17 fps)

Pond 3P: CB (new)



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Type III 24-hr 2-Year Storm Rainfall=3.20"

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Summary for Pond 4P: Doghouse MH (new)

[79] Warning: Submerged Pond 1P Primary device # 1 INLET by 0.33' [79] Warning: Submerged Pond 3P Primary device # 1 OUTLET by 0.71'

Inflow Area = 4.034 ac, 61.05% Impervious, Inflow Depth > 1.26" for 2-Year Storm event

Inflow = 2.64 cfs @ 12.11 hrs, Volume= 0.424 af

Outflow = 2.64 cfs @ 12.11 hrs, Volume= 0.424 af, Atten= 0%, Lag= 0.0 min

Primary = 2.64 cfs @ 12.11 hrs, Volume= 0.424 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Peak Elev= 96.35' @ 12.11 hrs

Flood Elev= 103.97'

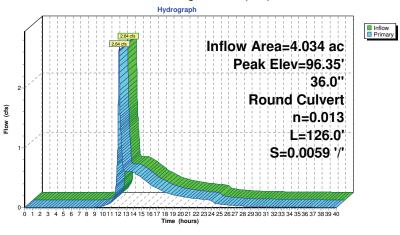
Device Routing Invert Outlet Devices

Primary 95.70' **36.0" Round 36" RCP**

L= 126.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 95.70' / 94.96' S= 0.0059 '/' Cc= 0.900 n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf

Primary OutFlow Max=2.63 cfs @ 12.11 hrs HW=96.35' (Free Discharge) 1=36" RCP (Barrel Controls 2.63 cfs @ 3.54 fps)

Pond 4P: Doghouse MH (new)



Type III 24-hr 2-Year Storm Rainfall=3.20"

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Summary for Link DP-1: South Wetland

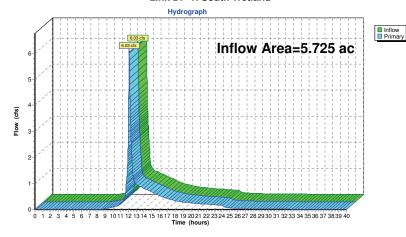
Inflow Area = 5.725 ac, 52.55% Impervious, Inflow Depth > 1.41" for 2-Year Storm event

Inflow = 6.03 cfs @ 12.10 hrs, Volume= 0.672 af

Primary = 6.03 cfs @ 12.10 hrs, Volume= 0.672 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Link DP-1: South Wetland



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Type III 24-hr 10-Year Storm Rainfall=4.70"

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Time span=0.00-40.00 hrs, dt=0.01 hrs, 4001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A: Syn Turf

Runoff Area=102,046 sf 100.00% Impervious Runoff Depth=4.46"

Tc=5.0 min CN=98 Runoff=11.15 cfs 0.871 af

Subcatchment B: South D-Zone & S&W Runoff Area=73,634 sf 32.24% Impervious Runoff Depth=3.09" Flow Length=219' Tc=6.4 min CN=85 Runoff=6.00 cfs 0.436 af

Subcatchment C: East Perimeter Runoff Area=26,551 sf 19.75% Impervious Runoff Depth=2.90"

Flow Length=655' Tc=8.7 min CN=83 Runoff=1.89 cfs 0.148 af

Subcatchment D: North Perimeter

Runoff Area=47,135 sf 0.00% Impervious Runoff Depth=2.55"
Flow Length=499' Tc=7.2 min CN=79 Runoff=3.10 cfs 0.230 af

Pond 1P: YD Peak Elev=96.59' Inflow=1.89 cfs 0.148 af

36.0" Round Culvert n=0.013 L=55.0' S=0.0058 '/' Outflow=1.89 cfs 0.148 af

Pond 2P: Turf Field Base Peak Elev=103.76' Storage=23,136 cf Inflow=11.15 cfs 0.871 af

Outflow=2.10 cfs 0.514 af

Pond 3P: CB (new) Peak Elev=98.09' Inflow=3.82 cfs 0.743 af

18.0" Round Culvert n=0.013 L=137.0' S=0.0100 '/' Outflow=3.82 cfs 0.743 af

Pond 4P: Doghouse MH (new) Peak Elev=96.67' Inflow=5.70 cfs 0.891 af

36.0" Round Culvert n=0.013 L=126.0' S=0.0059 '/' Outflow=5.70 cfs 0.891 af

 Link DP-1: South Wetland
 Inflow=11.48 cfs 1.326 af

 Primary=11.48 cfs 1.326 af

Total Runoff Area = 5.725 ac Runoff Volume = 1.684 af Average Runoff Depth = 3.53" 47.45% Pervious = 2.717 ac 52.55% Impervious = 3.008 ac

Type III 24-hr 10-Year Storm Rainfall=4.70"

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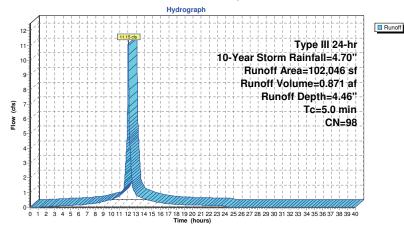
Summary for Subcatchment A: Syn Turf

11.15 cfs @ 12.07 hrs, Volume= 0.871 af, Depth= 4.46" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Storm Rainfall=4.70"

	Α	rea (sf)	CN I	Description						
4	1	02,046	98 3	Synthetic Turf						
	102,046 100.00% Impervious Are					urea				
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.0					Direct Entry, Min. Tc				

Subcatchment A: Syn Turf



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Type III 24-hr 10-Year Storm Rainfall=4.70" Printed 2/20/2019

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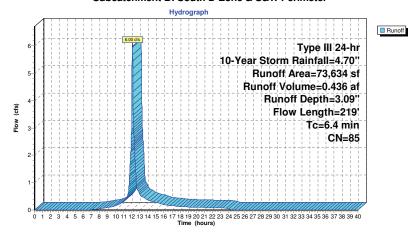
Summary for Subcatchment B: South D-Zone & S&W Perimeter

6.00 cfs @ 12.09 hrs, Volume= 0.436 af, Depth= 3.09" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Storm Rainfall=4.70"

			ON 5			
_	A	rea (sf)	CN E	Description		
*		20,920	98 F	Running Tra	ack	
*		403	98 F	Roof		
*		2.419	98 A	Asphalt Pav	/ement	
		49,892				Fair, HSG C
		73,634	85 V	Veighted A	verage	
		49,892	6	7.76% Per	vious Area	
		23,742	3	32.24% Imp	ervious Ar	ea
		,				
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
	5.6	60	0.0290	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.2	32	0.0390	3.18		Shallow Concentrated Flow,
		-				Unpaved Kv= 16.1 fps
	0.6	127	0.0420	3.30		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
_	6.4	219	Total			

Subcatchment B: South D-Zone & S&W Perimeter



Type III 24-hr 10-Year Storm Rainfall=4.70"

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Summary for Subcatchment C: East Perimeter

Runoff = 1.89 cfs @ 12.12 hrs, Volume= 0.148 af, Depth= 2.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Storm Rainfall=4.70"

A	rea (sf)	CN D	escription		
*	5,244	98 A	sphalt Pav	/ement	
	21,307	79 5	0-75% Gra	ass cover, F	Fair, HSG C
	26,551	83 V	Veighted A	verage	
	21,307	8	0.25% Per	vious Area	
	5,244	1	9.75% Imp	ervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.2	60	0.0230	0.16		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
1.8	396	0.0050	3.72	4.57	Pipe Channel, 15" RCP
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Concrete pipe, bends & connections
0.2	56	0.0050	5.09	16.00	Pipe Channel, 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
0.5	143	0.0050	5.09	16.00	Pipe Channel, 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
8.7	655	Total			

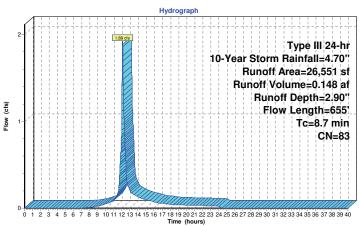
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Type III 24-hr 10-Year Storm Rainfall=4.70" Printed 2/20/2019

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Subcatchment C: East Perimeter





Type III 24-hr 10-Year Storm Rainfall=4.70"

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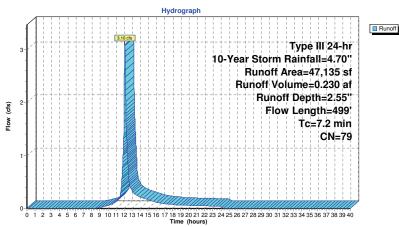
Summary for Subcatchment D: North Perimeter

0.230 af, Depth= 2.55" Runoff 3.10 cfs @ 12.10 hrs, Volume=

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Storm Rainfall=4.70"

Α	rea (sf)	CN E	Description		
	47,135	79 5	0-75% Gra	ass cover, I	Fair, HSG C
	47,135	1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.5	60	0.0300	0.18		Sheet Flow,
1.6	404	0.0050	4.20	7.43	Grass: Short n= 0.150 P2= 3.20" Pipe Channel, 18" HDPE Collector Pipe 18.0" Round Area= 1.8 sf Perim= 4.7" r= 0.38' n= 0.013 Corrugated PE, smooth interior
0.1	35	0.0050	4.20	7.43	Pipe Channel, 18" HDPE 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013 Corrugated PE, smooth interior
7.2	499	Total			

Subcatchment D: North Perimeter



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Type III 24-hr 10-Year Storm Rainfall=4.70" Printed 2/20/2019

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Summary for Pond 1P: YD

0.610 ac, 19.75% Impervious, Inflow Depth = 2.90" for 10-Year Storm event Inflow Area =

Inflow 1.89 cfs @ 12.12 hrs, Volume= 0.148 af

Outflow 1.89 cfs @ 12.12 hrs, Volume= 0.148 af, Atten= 0%, Lag= 0.0 min

Primary 1.89 cfs @ 12.12 hrs, Volume= 0.148 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

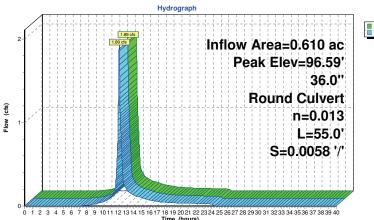
Peak Elev= 96.59' @ 12.12 hrs

Flood Elev= 103.97'

Device	Routing	Invert	Outlet Devices	
#1	Primary	96.02'	36.0" Round 36" RCP	
			L= 55.0' RCP, sq.cut end projecting, Ke= 0.500	
			Inlet / Outlet Invert= 96.02' / 95.70' S= 0.0058 '/' Cc= 0.900	
			n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf	

Primary OutFlow Max=1.88 cfs @ 12.12 hrs HW=96.58' (Free Discharge) 136" RCP (Barrel Controls 1.88 cfs @ 3.09 fps)

Pond 1P: YD





Type III 24-hr 10-Year Storm Rainfall=4.70" Printed 2/20/2019

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Summary for Pond 2P: Turf Field Base

2.343 ac,100.00% Impervious, Inflow Depth = 4.46" for 10-Year Storm event Inflow Area =

Inflow 11.15 cfs @ 12.07 hrs, Volume= 0.871 af Outflow 2.10 cfs @ 12.50 hrs, Volume= 0.514 af, Atten= 81%, Lag= 25.6 min

Primary 2.10 cfs @ 12.50 hrs, Volume= 0.514 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Peak Elev= 103.76' @ 12.50 hrs Surf.Area= 102,046 sf Storage= 23,136 cf

Plug-Flow detention time= 322.4 min calculated for 0.513 af (59% of inflow)

Center-of-Mass det. time= 210.6 min (958.7 - 748.1)

Volume	Inv	ert Ava	il.Storage	Storage Descrip				
#1	103.0	00'	45,921 cf	Custom Stage Data (Prismatic) Listed below (Recalc)				
Elevation (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
103.0 103.5 104.5	50	102,046 102,046 102,046	0.0 30.0 30.0	0 15,307 30,614	0 15,307 45,921			
Device	Routing	In	vert Out	let Devices				
#1	Primary	97	L= 3	18.0" Round 18" HDPE L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 97.28' / 97.10' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf				
#2 Device 1		1 103	3.50' 4.0'	' Vert. Orifice/Gra	ate X 17.00 C= 0).600		

Primary OutFlow Max=2.10 cfs @ 12.50 hrs HW=103.76' (Free Discharge)
1=18" HDPE (Passes 2.10 cfs of 16.07 cfs potential flow)
2=Orifice/Grate (Orifice Controls 2.10 cfs @ 1.72 fps)

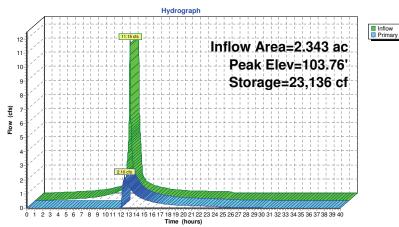
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Pond 2P: Turf Field Base



Type III 24-hr 10-Year Storm Rainfall=4.70"

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Summary for Pond 3P: CB (new)

[79] Warning: Submerged Pond 2P Primary device # 1 INLET by 0.81'

Inflow Area = 3.425 ac, 68.40% Impervious, Inflow Depth > 2.60" for 10-Year Storm event

3.82 cfs @ 12.13 hrs, Volume= 0.743 af Inflow

0.743 af, Atten= 0%, Lag= 0.0 min Outflow 3.82 cfs @ 12.13 hrs, Volume=

3.82 cfs @ 12.13 hrs, Volume= 0.743 af Primary

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Peak Elev= 98.09' @ 12.13 hrs

Flood Elev= 103.70'

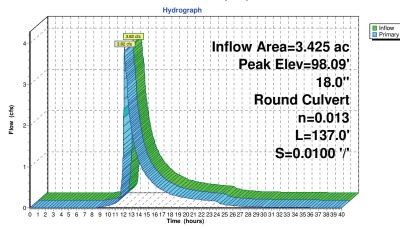
Device	Routing	Invert	Outlet Devices			
#1	Primary	97.01'	18.0" Round 18" HDPE L= 137.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 97.01' / 95.64' S= 0.0100' Cc= 0.900			

Primary OutFlow Max=3.82 cfs @ 12.13 hrs HW=98.09' (Free Discharge)

1=18" HDPE (Inlet Controls 3.82 cfs @ 2.80 fps)

Pond 3P: CB (new)

n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf



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Summary for Pond 4P: Doghouse MH (new)

[81] Warning: Exceeded Pond 1P by 0.13' @ 12.51 hrs

[79] Warning: Submerged Pond 3P Primary device # 1 OUTLET by 1.03'

Inflow Area = 4.034 ac, 61.05% Impervious, Inflow Depth > 2.65" for 10-Year Storm event

Inflow 5.70 cfs @ 12.13 hrs, Volume= 0.891 af

Outflow = 5.70 cfs @ 12.13 hrs, Volume= 0.891 af, Atten= 0%, Lag= 0.0 min

5.70 cfs @ 12.13 hrs, Volume= 0.891 af Primary =

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

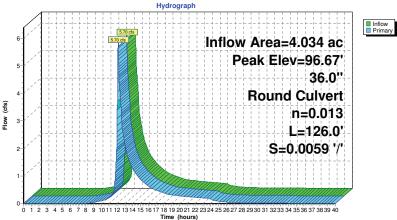
Peak Elev= 96.67' @ 12.13 hrs

Flood Elev= 103.97'

Device	Routing	Invert	Outlet Devices
#1	Primary	95.70'	36.0" Round 36" RCP
			L= 126.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 95.70' / 94.96' S= 0.0059 '/' Cc= 0.900
			n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf

Primary OutFlow Max=5.69 cfs @ 12.13 hrs HW=96.67' (Free Discharge) 1=36" RCP (Barrel Controls 5.69 cfs @ 4.30 fps)

Pond 4P: Doghouse MH (new)



Type III 24-hr 10-Year Storm Rainfall=4.70"

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Summary for Link DP-1: South Wetland

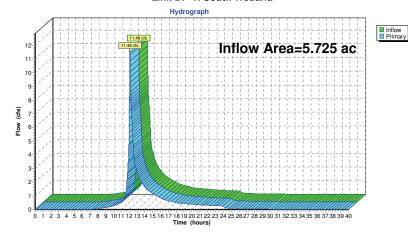
Inflow Area = 5.725 ac, 52.55% Impervious, Inflow Depth > 2.78" for 10-Year Storm event

Inflow = 11.48 cfs @ 12.11 hrs, Volume= 1.326 af

Primary = 11.48 cfs @ 12.11 hrs, Volume= 1.326 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Link DP-1: South Wetland



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Type III 24-hr 25-Year Storm Rainfall=5.50" Printed 2/20/2019

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Time span=0.00-40.00 hrs, dt=0.01 hrs, 4001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A: Syn Turf Runoff Area=102,046 sf 100.00% Impervious Runoff Depth=5.26"

Tc=5.0 min CN=98 Runoff=13.07 cfs 1.027 af

Subcatchment B: South D-Zone & S&W Runoff Area=73,634 sf 32.24% Impervious Runoff Depth=3.83" Flow Length=219' Tc=6.4 min CN=85 Runoff=7.38 cfs 0.540 af

Subcatchment C: East Perimeter Runoff Area=26,551 sf 19.75% Impervious Runoff Depth=3.63"

Subcatchment D: North Perimeter Runoff Area=47,135 sf 0.00% Impervious Runoff Depth=3.24"

Flow Length=499' Tc=7.2 min CN=79 Runoff=3.94 cfs 0.292 af

Pond 1P: YD Peak Elev=96.65' Inflow=2.35 cfs 0.184 af

36.0" Round Culvert n=0.013 L=55.0' S=0.0058 '/' Outflow=2.35 cfs 0.184 af

Flow Length=655' Tc=8.7 min CN=83 Runoff=2.35 cfs 0.184 af

Pond 2P: Turf Field Base Peak Elev=103.85' Storage=25,937 cf Inflow=13.07 cfs 1.027 af

Outflow=3.03 cfs 0.669 af

Pond 3P: CB (new) Peak Elev=98.52' Inflow=5.87 cfs 0.961 af

18.0" Round Culvert n=0.013 L=137.0' S=0.0100 '/' Outflow=5.87 cfs 0.961 af

Pond 4P: Doghouse MH (new) Peak Elev=96.88' Inflow=8.21 cfs 1.146 af

36.0" Round Culvert n=0.013 L=126.0' S=0.0059 '/' Outflow=8.21 cfs 1.146 af

Link DP-1: South Wetland Inflow=15.31 cfs 1.686 af

Primary=15.31 cfs 1.686 af

Total Runoff Area = 5.725 ac Runoff Volume = 2.044 af 47.45% Pervious = 2.717 ac 52.55% Impervious = 3.008 ac

Type III 24-hr 25-Year Storm Rainfall=5.50"

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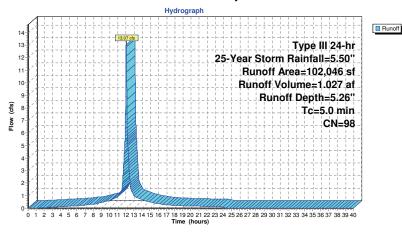
Summary for Subcatchment A: Syn Turf

13.07 cfs @ 12.07 hrs, Volume= 1.027 af, Depth= 5.26" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Storm Rainfall=5.50"

	Α	rea (sf)	CN [escription						
3	1	02,046	98 3	Synthetic Turf						
102,046 100.00% Impervious					pervious A	urea				
	Tc	Length	Slope		Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.0					Direct Entry, Min. Tc				

Subcatchment A: Syn Turf



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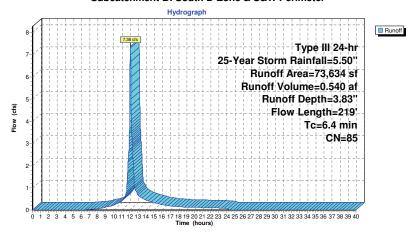
Summary for Subcatchment B: South D-Zone & S&W Perimeter

7.38 cfs @ 12.09 hrs, Volume= 0.540 af, Depth= 3.83" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Storm Rainfall=5.50"

	Д	rea (sf)	CN [Description		
*		20.920	98 F	Running Tra	ack	
*		403		Roof		
*		2,419	98 A	Asphalt Pav	vement	
		49,892	79 5	60-75% Gra	ass cover, I	Fair, HSG C
		73,634	85 V	Veighted A	verage	
		49,892	6	7.76% Per	rvious Area	
		23,742	3	32.24% Imp	pervious Ar	ea
	Tc	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.6	60	0.0290	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.2	32	0.0390	3.18		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	0.6	127	0.0420	3.30		Shallow Concentrated Flow,
_						Unpaved Kv= 16.1 fps
	6.4	219	Total			

Subcatchment B: South D-Zone & S&W Perimeter



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Summary for Subcatchment C: East Perimeter

Runoff = 2.35 cfs @ 12.12 hrs, Volume= 0.184 af, Depth= 3.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Storm Rainfall=5.50"

A	rea (sf)	CN D	escription		
*	5,244	98 A	sphalt Pav	rement	
	21,307	79 5	0-75% Gra	ass cover, I	Fair, HSG C
	26,551	83 V	Veighted A	verage	
	21,307	8	0.25% Per	vious Area	
	5,244	1	9.75% Imp	ervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.2	60	0.0230	0.16		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
1.8	396	0.0050	3.72	4.57	Pipe Channel, 15" RCP
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Concrete pipe, bends & connections
0.2	56	0.0050	5.09	16.00	Pipe Channel, 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
0.5	143	0.0050	5.09	16.00	Pipe Channel, 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
8.7	655	Total			

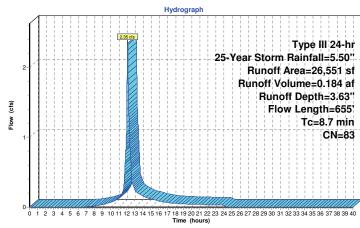
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Subcatchment C: East Perimeter





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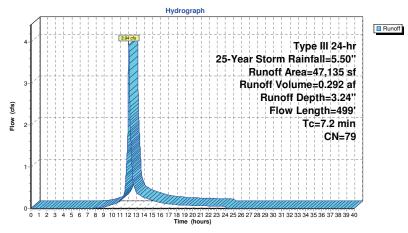
Summary for Subcatchment D: North Perimeter

0.292 af, Depth= 3.24" Runoff 3.94 cfs @ 12.10 hrs, Volume=

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Storm Rainfall=5.50"

Α	rea (sf)	CN [Description		
	47,135	79 5	0-75% Gra	ass cover, I	Fair, HSG C
	47,135	1	100.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.5	60	0.0300	0.18		Sheet Flow,
1.6	404	0.0050	4.20	7.43	Grass: Short n= 0.150 P2= 3.20" Pipe Channel, 18" HDPE Collector Pipe 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013 Corrugated PE. smooth interior
0.1	35	0.0050	4.20	7.43	,
7.2	499	Total			

Subcatchment D: North Perimeter



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Summary for Pond 1P: YD

0.610 ac, 19.75% Impervious, Inflow Depth = 3.63" for 25-Year Storm event Inflow Area =

Inflow 2.35 cfs @ 12.12 hrs, Volume= 0.184 af

Outflow 2.35 cfs @ 12.12 hrs, Volume= 0.184 af, Atten= 0%, Lag= 0.0 min

2.35 cfs @ 12.12 hrs, Volume= Primary 0.184 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

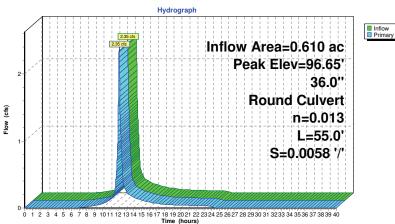
Peak Elev= 96.65' @ 12.12 hrs

Flood Elev= 103.97'

Device	Routing	Invert	Outlet Devices
#1	Primary	96.02'	36.0" Round 36" RCP
			L= 55.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 96.02' / 95.70' S= 0.0058 '/' Cc= 0.900
			n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf

Primary OutFlow Max=2.34 cfs @ 12.12 hrs HW=96.65' (Free Discharge) 1=36" RCP (Barrel Controls 2.34 cfs @ 3.26 fps)

Pond 1P: YD



Type III 24-hr 25-Year Storm Rainfall=5.50"

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Summary for Pond 2P: Turf Field Base

2.343 ac,100.00% Impervious, Inflow Depth = 5.26" for 25-Year Storm event Inflow Area = Inflow 13.07 cfs @ 12.07 hrs, Volume= 1.027 af Outflow 3.03 cfs @ 12.45 hrs, Volume= 0.669 af, Atten= 77%, Lag= 22.6 min Primary 3.03 cfs @ 12.45 hrs, Volume= 0.669 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Peak Elev= 103.85' @ 12.45 hrs Surf.Area= 102,046 sf Storage= 25,937 cf

Plug-Flow detention time= 290.2 min calculated for 0.669 af (65% of inflow) Center-of-Mass det. time= 187.5 min (933.1 - 745.5)

Avail Storage Storage Description

volume	invert /	avali.Storagi	e Storage Descri	iption		
#1	103.00'	45,921 c	f Custom Stage	Data (Prismatic)	Listed below (Recalc)	
Elevation (feet)	Surf.Ar (sq		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
103.00 103.50	102,0 102.0		0 15.307	0 15.307		
104.50	102,0		30,614	45,921		
Device R	outina	Invert O	utlet Devices			

Primary 97.28' 18.0" Round 18" HDPE L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 97.28° / 97.10° S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf 103.50' **4.0" Vert. Orifice/Grate X 17.00** C= 0.600 #2 Device 1

Primary OutFlow Max=3.04 cfs @ 12.45 hrs HW=103.85' (Free Discharge) 1=18" HDPE (Passes 3.04 cfs of 16.20 cfs potential flow)

2=Orifice/Grate (Orifice Controls 3.04 cfs @ 2.05 fps)

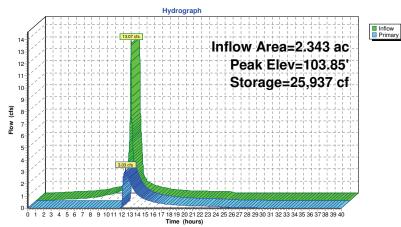
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Pond 2P: Turf Field Base



Type III 24-hr 25-Year Storm Rainfall=5.50"

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Summary for Pond 3P: CB (new)

[79] Warning: Submerged Pond 2P Primary device # 1 INLET by 1.24'

Inflow Area = 3.425 ac, 68.40% Impervious, Inflow Depth > 3.37" for 25-Year Storm event

Inflow 5.87 cfs @ 12.13 hrs, Volume= 0.961 af

Outflow 5.87 cfs @ 12.13 hrs, Volume= 0.961 af, Atten= 0%, Lag= 0.0 min

5.87 cfs @ 12.13 hrs, Volume= Primary 0.961 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

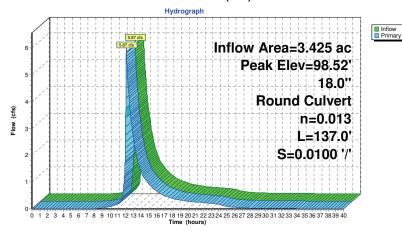
Peak Elev= 98.52' @ 12.13 hrs

Flood Elev= 103.70'

Device	Routing	Invert	Outlet Devices
	Primary		18.0" Round 18" HDPE L= 137.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 97.01' / 95.64' S= 0.0100 '/' Cc= 0.900
#1	Primary	97.01'	L= 137.0' CPP, projecting, no headwall, Ke= 0.900

Primary OutFlow Max=5.86 cfs @ 12.13 hrs HW=98.52' (Free Discharge) 1=18" HDPE (Inlet Controls 5.86 cfs @ 3.31 fps)

Pond 3P: CB (new)



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Type III 24-hr 25-Year Storm Rainfall=5.50"

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Summary for Pond 4P: Doghouse MH (new)

[81] Warning: Exceeded Pond 1P by 0.23' @ 12.14 hrs

[79] Warning: Submerged Pond 3P Primary device # 1 OUTLET by 1.24'

Inflow Area = 4.034 ac, 61.05% Impervious, Inflow Depth > 3.41" for 25-Year Storm event

Inflow 8.21 cfs @ 12.13 hrs, Volume= 1.146 af

Outflow = 8.21 cfs @ 12.13 hrs, Volume= 1.146 af, Atten= 0%, Lag= 0.0 min

8.21 cfs @ 12.13 hrs, Volume= 1.146 af Primary =

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

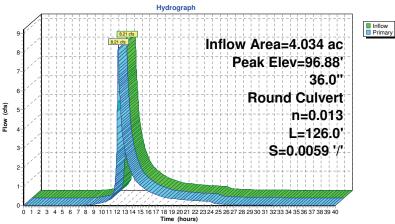
Peak Elev= 96.88' @ 12.13 hrs

Flood Elev= 103.97'

Device	Routing	Invert	Outlet Devices
#1	Primary	95.70'	36.0" Round 36" RCP
			L= 126.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 95.70' / 94.96' S= 0.0059 '/' Cc= 0.900
			n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf

Primary OutFlow Max=8.20 cfs @ 12.13 hrs HW=96.88' (Free Discharge) 1=36" RCP (Barrel Controls 8.20 cfs @ 4.68 fps)

Pond 4P: Doghouse MH (new)



Type III 24-hr 25-Year Storm Rainfall=5.50"

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Summary for Link DP-1: South Wetland

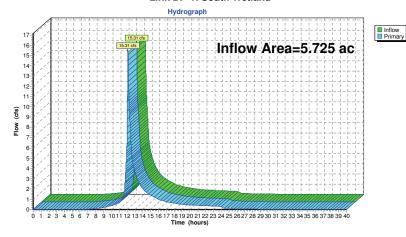
5.725 ac, 52.55% Impervious, Inflow Depth > 3.53" for 25-Year Storm event Inflow Area =

Inflow 15.31 cfs @ 12.11 hrs, Volume= 1.686 af

15.31 cfs @ 12.11 hrs, Volume= 1.686 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Link DP-1: South Wetland



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Type III 24-hr 100-Year Storm Rainfall=6.90"

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Time span=0.00-40.00 hrs, dt=0.01 hrs, 4001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A: Syn Turf Runoff Area=102.046 sf 100.00% Impervious Runoff Depth=6.66"

Tc=5.0 min CN=98 Runoff=16.43 cfs 1.300 af

Subcatchment B: South D-Zone & S&W Runoff Area=73,634 sf 32.24% Impervious Runoff Depth=5.16" Flow Length=219' Tc=6.4 min CN=85 Runoff=9.81 cfs 0.726 af

Subcatchment C: East Perimeter Runoff Area=26,551 sf 19.75% Impervious Runoff Depth=4.93"

Flow Length=655' Tc=8.7 min CN=83 Runoff=3.16 cfs 0.251 af

Subcatchment D: North Perimeter Runoff Area=47,135 sf 0.00% Impervious Runoff Depth=4.49"

Flow Length=499' Tc=7.2 min CN=79 Runoff=5.43 cfs 0.405 af

Pond 1P: YD Peak Elev=96.76' Inflow=3.16 cfs 0.251 af 36.0" Round Culvert n=0.013 L=55.0' S=0.0058 '/' Outflow=3.16 cfs 0.251 af

Peak Elev=104.01' Storage=31,049 cf Inflow=16.43 cfs 1.300 af Pond 2P: Turf Field Base

Outflow=4.21 cfs 0.942 af

Peak Elev=99.49' Inflow=8.83 cfs 1.347 af Pond 3P: CB (new)

18.0" Round Culvert n=0.013 L=137.0' S=0.0100 '/' Outflow=8.83 cfs 1.347 af

Peak Elev=97.17' Inflow=11.99 cfs 1.598 af Pond 4P: Doghouse MH (new)

36.0" Round Culvert n=0.013 L=126.0' S=0.0059 '/' Outflow=11.99 cfs 1.598 af

Link DP-1: South Wetland Inflow=21.58 cfs 2.325 af

Primary=21.58 cfs 2.325 af

Total Runoff Area = 5.725 ac Runoff Volume = 2.683 af Average Runoff Depth = 5.62" 47.45% Pervious = 2.717 ac 52.55% Impervious = 3.008 ac

Type III 24-hr 100-Year Storm Rainfall=6.90"

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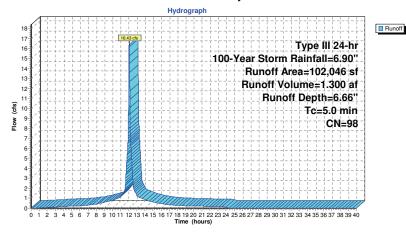
Summary for Subcatchment A: Syn Turf

16.43 cfs @ 12.07 hrs, Volume= 1.300 af, Depth= 6.66" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Storm Rainfall=6.90"

	Α	rea (sf)	CN I	Description				
4	1	02,046	98 3	Synthetic Turf				
	1	02,046	02,046 100.00% Impervious A			urea		
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	5.0					Direct Entry, Min. Tc		

Subcatchment A: Syn Turf



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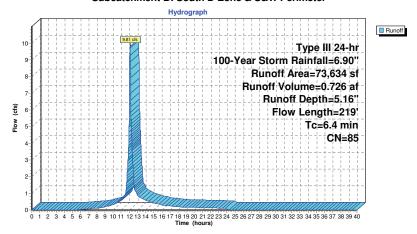
Summary for Subcatchment B: South D-Zone & S&W Perimeter

9.81 cfs @ 12.09 hrs, Volume= 0.726 af, Depth= 5.16" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Storm Rainfall=6.90"

	Α	rea (sf)	CN [Description		
-	+	20.920	98 F	Running Tra	ack	
	+	403		Roof		
	+	2,419	98 <i>F</i>	Asphalt Pav	vement	
		49,892	79 5	60-75% Gra	ass cover, F	Fair, HSG C
		73,634	85 V	Veighted A	verage	
		49,892	6	7.76% Per	rvious Area	
		23,742	3	32.24% Imp	pervious Ar	ea
	Tc	Length	Slope		Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.6	60	0.0290	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.2	32	0.0390	3.18		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	0.6	127	0.0420	3.30		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	6.4	219	Total			

Subcatchment B: South D-Zone & S&W Perimeter



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Summary for Subcatchment C: East Perimeter

Runoff 3.16 cfs @ 12.12 hrs, Volume= 0.251 af, Depth= 4.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Storm Rainfall=6.90"

^	roo (cf)	CN D	escription		
	rea (sf)				
*	5,244		sphalt Pav		
	21,307	79 5	0-75% Gra	ass cover, l	Fair, HSG C
	26,551	83 V	Veighted A	verage	
	21.307			vious Area	
	5,244	-		ervious Ar	
	5,244	'	5.7576 IIIIp	oci vious Air	ca
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Boompaon
6.2	60	0.0230	0.16	(013)	Chast Flaur
0.2	60	0.0230	0.16		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.20"
1.8	396	0.0050	3.72	4.57	
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Concrete pipe, bends & connections
0.2	56	0.0050	5.09	16.00	Pipe Channel, 24" RCP
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
0.5	143	0.0050	5.09	16.00	Pipe Channel, 24" RCP
0.0	0	0.0000	0.00	.0.00	24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.013 Concrete pipe, bends & connections
		T			11- 0.013 Contracte pipe, belias & confidentions
8.7	655	Total			

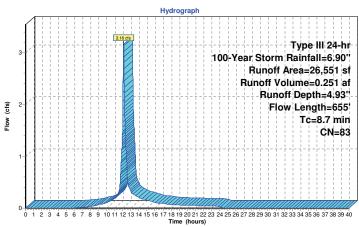
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Subcatchment C: East Perimeter





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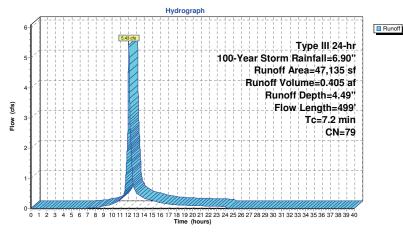
Summary for Subcatchment D: North Perimeter

Runoff = 5.43 cfs @ 12.10 hrs, Volume= 0.405 af, Depth= 4.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Storm Rainfall=6.90"

A	rea (sf)	CN D	escription			
	47,135	79 5	79 50-75% Grass cover, Fair, HSG C			
	47,135	1	00.00% Pe	ervious Are	a	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
5.5	60	0.0300	0.18		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.20"	
1.6	404	0.0050	4.20	7.43		
					18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'	
0.1	35	0.0050	4.20	7.43	n= 0.013 Corrugated PE, smooth interior Pipe Channel, 18" HDPE	
					18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'	
					n= 0.013 Corrugated PE, smooth interior	
7.2	499	Total				

Subcatchment D: North Perimeter



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Summary for Pond 1P: YD

 Inflow Area = Inflow Depth = 4.93" for 100-Year Storm event = Inflow = Inflow Depth = 4.93" for 100-Year Storm event = Inflow = Inflow Depth = 4.93" for 100-Year Storm event = Inflow Depth = Inflow

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs Peak Elev= 96.76' @ 12.12 hrs

Flood Elev= 103.97'

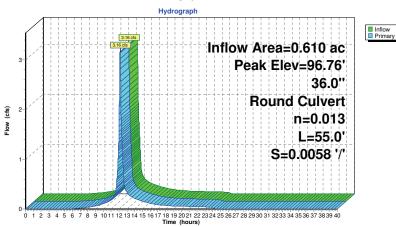
 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 96.02'
 36.0"
 Round 36"
 RCP

L= 55.0° RCP, sq.cut end projecting, Ke= 0.500
Inlet / Outlet Invert= 96.02′ / 95.70° S= 0.0058′ Cc= 0.900
n= 0.013 Concrete pipe, bends & connections, Flow Area= 7.07 sf

Primary OutFlow Max=3.15 cfs @ 12.12 hrs HW=96.76' (Free Discharge)
1=36" RCP (Barrel Controls 3.15 cfs @ 3.50 fps)

Pond 1P: YD



Type III 24-hr 100-Year Storm Rainfall=6.90"

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Summary for Pond 2P: Turf Field Base

2.343 ac,100.00% Impervious, Inflow Depth = 6.66" for 100-Year Storm event Inflow Area =

Inflow 16.43 cfs @ 12.07 hrs, Volume= 1.300 af

Outflow 4.21 cfs @ 12.42 hrs, Volume= 0.942 af, Atten= 74%, Lag= 20.9 min

Primary 4.21 cfs @ 12.42 hrs, Volume= 0.942 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Peak Elev= 104.01' @ 12.42 hrs Surf.Area= 102,046 sf Storage= 31,049 cf

Plug-Flow detention time= 259.0 min calculated for 0.942 af (72% of inflow)

Center-of-Mass det. time= 166.9 min (909.1 - 742.2)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	103.0	00'	45,921 cf	Custom Stage	Data (Prismatic) L	isted below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
103.0 103.5 104.5	50	102,046 102,046 102,046	0.0 30.0 30.0	0 15,307 30,614	0 15,307 45,921	
Device	Routing	Ir	vert Out	let Devices		
#1	Primary	97	L= 3 Inle	t / Outlet Invert= 9	cting, no headwall, 97.28' / 97.10' S=	, Ke= 0.900 0.0051 '/' Cc= 0.900 or, Flow Area= 1.77 sf
#2	Device 1	l 103	3.50' 4.0' '	Vert. Orifice/Gra	ate X 17.00 C= 0	.600

Primary OutFlow Max=4.21 cfs @ 12.42 hrs HW=104.01' (Free Discharge)
1=18" HDPE (Passes 4.21 cfs of 16.43 cfs potential flow)
2=Orifice/Grate (Orifice Controls 4.21 cfs @ 2.84 fps)

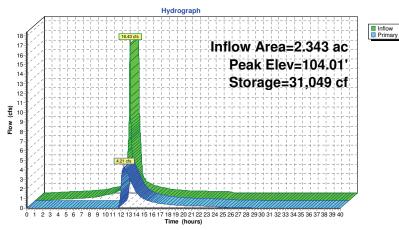
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Pond 2P: Turf Field Base



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Summary for Pond 3P: CB (new)

[79] Warning: Submerged Pond 2P Primary device # 1 INLET by 2.21'

Inflow Area = 3.425 ac, 68.40% Impervious, Inflow Depth > 4.72" for 100-Year Storm event

8.83 cfs @ 12.12 hrs, Volume= 1.347 af Inflow

Outflow 8.83 cfs @ 12.12 hrs, Volume= 1.347 af, Atten= 0%, Lag= 0.0 min

8.83 cfs @ 12.12 hrs, Volume= Primary 1.347 af

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Peak Elev= 99.49' @ 12.12 hrs

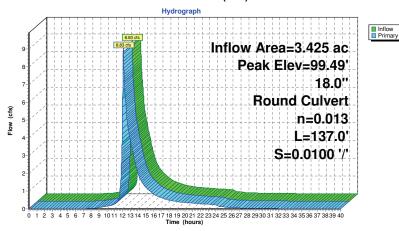
Flood Elev= 103.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	97.01'	18.0" Round 18" HDPE
			L= 137.0' CPP, projecting, no headwall, Ke= 0.900

n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf Primary OutFlow Max=8.82 cfs @ 12.12 hrs HW=99.49' (Free Discharge)

1=18" HDPE (Inlet Controls 8.82 cfs @ 4.99 fps)

Pond 3P: CB (new)



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Type III 24-hr 100-Year Storm Rainfall=6.90"

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Summary for Pond 4P: Doghouse MH (new)

[81] Warning: Exceeded Pond 1P by 0.40' @ 12.11 hrs

[79] Warning: Submerged Pond 3P Primary device # 1 INLET by 0.15'

Inflow Area = 4.034 ac, 61.05% Impervious, Inflow Depth > 4.75" for 100-Year Storm event

Inflow 11.99 cfs @ 12.12 hrs, Volume= 1.598 af

1.598 af, Atten= 0%, Lag= 0.0 min Outflow = 11.99 cfs @ 12.12 hrs, Volume=

11.99 cfs @ 12.12 hrs, Volume= 1.598 af Primary =

Routing by Stor-Ind method, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

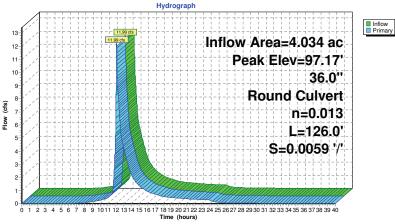
Peak Elev= 97.17' @ 12.12 hrs

Flood Elev= 103.97'

Device	Routing	Invert	Outlet Devices
#1	Primary	95.70'	36.0" Round 36" RCP
			L= 126.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 95.70' / 94.96' S= 0.0059 '/' Cc= 0.900
			n= 0.013 Concrete pipe, bends & connections. Flow Area= 7.07 sf

Primary OutFlow Max=11.97 cfs @ 12.12 hrs HW=97.16' (Free Discharge) 1=36" RCP (Barrel Controls 11.97 cfs @ 5.11 fps)

Pond 4P: Doghouse MH (new)



Type III 24-hr 100-Year Storm Rainfall=6.90"

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Summary for Link DP-1: South Wetland

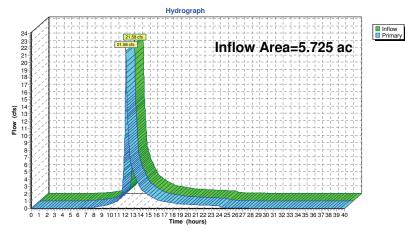
5.725 ac, 52.55% Impervious, Inflow Depth > 4.87" for 100-Year Storm event 21.58 cfs @ 12.10 hrs, Volume= 2.325 af 21.58 cfs @ 12.10 hrs, Volume= 2.325 af, Atten= 0%, Lag= 0.0 min Inflow Area =

Inflow

Primary =

Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, dt= 0.01 hrs

Link DP-1: South Wetland



University of Saint Joseph – Athletic Field Renovations

Stormwater Management Report

Stormwater System Maintenance

Appendix E

CONTENTS:

- 1. Stormwater Maintenance Narrative
- 2. Stormwater Maintenance Log



STORMWATER FACILITIES INSPECTION AND MAINTENANCE PLAN

University of Saint Joseph – Athletic Field Renovations

During construction activities, the maintenance of all stormwater measures will be the direct responsibility of the Contractor undertaking the work. All work shall conform to the terms and conditions of all relevant local, State and/or federal permits. After acceptance by the Owner, the maintenance of all stormwater management facilities, the establishment of any contract services required to implement the program, and the keeping of records and maintenance log book will be the responsibility of the University. Notwithstanding any other schedule noted below, general inspections should be conducted by facilities staff monthly, during, or immediately following rainfall events so that the function of the systems can be suitably observed.

Ditches and Swales

Open swales and ditches shall be inspected at a minimum on a quarterly basis, and before and after a major rainfall event to assure that debris and/or sediments do not reduce the effectiveness of the system. Debris noticed during an inspection shall be removed at that time, or within 24-hours of the inspection. Any sign of erosion or blockage shall be immediately repaired and stabilized to ensure the stability of the structure and proper function. Maintenance shall include, but not be limited to, mowing, trimming and removal vegetation in the ditches as required to prevent vegetation from blocking or diverting storm flows, removing vegetation and debris from the culverts, inlet and outlet structures.

Drainage Pipes and Culverts

Culverts and piped drainage systems shall be inspected on an annual basis to remove any obstructions to flow; remove accumulated sediments and debris at the inlet, at the outlet, and within the conduit; and to repair any erosion damage at the pipe inlet and outlet. Sediment should be removed when its level exceeds 20% of the pipe diameter. This may be accomplished by hydraulic flushing or any mechanical means; however, care should be taken to contain the sediment at the pipe outlet, and not flush the sediments into the wetland areas.

Inlet and Outlet Grates

Inlet and outlet grates are intended to trap and control floatables and debris within the stormwater system. The grates should be inspected on a quarterly basis, and after large storm events for build up of debris and other potentially detrimental material. Periodic maintenance of these features will be required to keep grates clear and prevent damage to either the grate itself or the attached structure.

Catch Basins

Inspect catch basins at least four times a year, and at the end of foliage and snow removal seasons. Remove sediments from catch basin at least four times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin. (Removal of sediments shall occur a minimum of once per year). Clean out must include the removal and legal disposal of accumulated sediments and debris at the bottom of the basin, at any inlet grates, at any inflow channels to the basin, and at any pipes between basins.

Stormwater Inspection and Maintenance Log

Site Name:	University of Saint Joseph	Location:	West Hartford, CT	Date of Inspection:	

ВМР	Quarterly Inspection tasks	Completed	Notes	Maintenance Required	Maintenance Complete
Slopes	Check for signs of erosion				
	Check for settlement/slumping				
Paved Areas	Check for sand build-up				
	Check edges for displacement/failure				
Catch Basins	Check sediment levels in sumps				
	Check grates				
	Check for debris/litter				
Culverts	Inspect structural integrity				
	Look for joint displacement				
	Inspect inlet and outlet structures				
	Check for sediment accumulation				